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GIBSON ASSOCIATES, P.A.

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## Letter of Transmittal

<b>Date</b> 4/20/07	<b>Project No.</b> SA7047
<b>Attention:</b> Mr. Mark J. Gibson	
<b>Re:</b> Traffic Engineering Assessment – R.E. Pierson Construction – SA# 7047	

**TO: Mr. Mark J. Gibson**  
R.E. Pierson Construction Company  
c/o Gibson Associates  
522 Sea Isle Boulevard  
Ocean View, NJ 08230

### WE ARE SENDING YOU:

☐ Reports ☐ Prints ☐ Plans ☐ Disks ☐ Specifications ☐ Copy of Letter ☐ Check

Copies	Date	No.	Description
10	4/20/07		Traffic Engineering Assessment – R.E. Pierson Construction – SA# 7047

### TRANSMITTED as checked below:

☐ For Approval ☐ As requested ☐ For review and comment


# Shropshire|Associates LLC

Traffic Engineering  
Noise & Air Evaluations  
Eminent Domain Consulting  
Transportation Planning  
Parking Studies  
Access Permitting  
Traffic Signal Design

662 MAIN STREET, SUITE B  
LUMBERTON, NJ 08048

DAVID R SHROPSHIRE, PE, PP  
A ANDREW FERANDA, PE, CME

PHONE  
609 714 0400  
FAX  
609 714 9944

April 20, 2007

R.E. Pierson Construction Company  
c/o Gibson Associates  
522 Sea Isle Boulevard  
Ocean View, New Jersey 08230

(via UPS)

Attn: Mark J. Gibson, P.L.S.

**Re: Traffic Engineering Assessment  
R.E. Pierson Construction  
Woodbine-Ocean View Road (CR 550)  
Dennis Township, Cape May County  
SA Project No. 7047**

Dear Mark:

In response to your request, a traffic engineering assessment has been prepared for the proposed R.E. Pierson construction facility located in Dennis Township, Cape May County, New Jersey (Figure 1). The proposal is for the development of construction facility that will include an asphalt plant, concrete plant, and recycling facility. These uses are proposed in addition to the existing sand mining operation on-site. The site is located along westbound Woodbine-Ocean View Road, west of its intersection with Corsons Tavern Road, with access being provided via two existing driveways along the roadway. The purpose of this assessment is to determine the amount of additional traffic to be generated by the proposed construction facility and analyze its impact on the adjacent roadway network.

## Existing Conditions

A field reconnaissance was conducted to determine the existing intersection and roadway characteristics in the vicinity of the proposed site. A brief description of the roadways and intersections is provided below.

In the vicinity of the site, **Woodbine-Ocean View Road (CR 550)** is a two-lane undivided roadway under the jurisdiction of Cape May County and classified<sup>1</sup> as a Rural Major Collector along the site frontage. Woodbine-Ocean View Road has a posted speed limit of 50 MPH and an approximate cartway width of 40 feet, including 8-foot shoulders in both directions and is designated as a Truck Route. For the purpose of this study, Woodbine-Ocean View Road is assumed to extend in a general east-west direction.

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<sup>1</sup> 2006 New Jersey Functional Classification Maps



**Corsons Tavern Road (CR 628)** is a two-lane undivided roadway that is classified as an Urban Collector and under the jurisdiction of Cape May County. Corsons Tavern Road has a posted speed limit of 45 MPH in the vicinity of its intersection with Woodbine-Ocean View Road and an approximate cartway width of 35 feet, including 7-foot shoulders in both directions. For the purpose of this study, Corsons Tavern Road is assumed to extend in a general north-south direction.

The four-legged **Woodbine-Ocean View Road/Corsons Tavern Road** intersection is controlled by a two-phase fully-actuated traffic signal with a 41 to 64-second variable cycle length. All approaches consist of a single lane providing for all possible movements.

The T-shaped **Woodbine-Ocean View Road/Site Access** intersections are stop-controlled along the southbound site access approaches. All approaches consist of a single lane providing for all possible movements.

### **Traffic Counts**

In April 2007, manual turning movement counts (MTMC) were conducted at the above-mentioned study intersections. This data was analyzed to determine the peak hour traffic volumes that coincide with the peak combined volumes of the roadway and proposed development, which typically occur during the weekday AM (6:00 to 9:00 AM) and weekday PM (1:00 to 4:00 PM) peak periods. These periods were based upon conversations with the applicant regarding the peak operation of the construction facility. Figure 2a illustrates the existing AM and PM peak hour volumes at the above-mentioned intersections.

Due to the proposed R.E. Pierson construction facility being in the vicinity of the New Jersey coastal areas, increased traffic volumes are anticipated along the adjacent roadway network during the summer season. In order to provide for a more conservative analysis of the study intersections and adjacent roadway network, summer traffic data was obtained from Cape May County for Woodbine-Ocean View Road between its intersections with Corsons Tavern Road and Route 9. This data was used to extrapolate seasonal adjustment factors for the weekday AM and PM peak hours. These factors of 1.50 and 1.96 respectively, were applied to the April 2007 traffic volumes to adjust for peak seasonal volumes in the vicinity of the site. Figure 2b indicates the seasonally adjusted AM and PM peak hour volumes at the study locations. It should be noted that these volumes are used as the basis for all Existing, No-Build, and Build scenario analyses discussed below.

### **Future Conditions**

The traffic resulting from the proposed R.E. Pierson construction facility will not affect the adjacent roadway network until it is fully built-out and occupied. It is estimated that the proposed development will be completed and occupied by 2009. Therefore, it can be expected that the existing traffic volumes will increase as result of other developments in the area of the site. Based on the New Jersey Functional Classification Maps, as well as the *Annual Background Growth Table* prepared by the New Jersey Department of Transportation, a 2.50% annual traffic growth will occur in the vicinity of the site. Figure 3 shows the projected 2009 No-Build volumes for the adjacent roadway network.



The amount of traffic to be generated by the proposed construction facility can best be determined by comparison with similar sites. Traffic counts were conducted at the existing R.E. Pierson construction facility in Bridgeport on Oak Grove Road. The existing facility in Bridgeport contains an asphalt plant, concrete plant, recycling facility, and sand plant and is similar in use to the proposed facility in Dennis Township. Table 1 indicates the amount of traffic that entered and exited during the weekday AM and PM peak hours at the existing R.E. Pierson facility in Bridgeport. The peak hours for the large vehicle traffic occurred between 8:00 and 9:00 AM, and 1:00 and 2:00 PM.

<b>Table 1 Bridgeport Facility Trip Generation</b>							
Development	Vehicle	AM Peak Hour			PM Peak Hour		
		In	Out	Total	In	Out	Total
Construction Facility	Large Vehicles	12	14	26	20	19	39
	Passenger Vehicles	15	3	18	3	3	6
	<b>Totals</b>	<b>27</b>	<b>17</b>	<b>44</b>	<b>23</b>	<b>22</b>	<b>45</b>

Based upon conversations with R.E. Pierson, April is typically not the peak month of operation for the existing facility in Bridgeport and the proposed facility in Dennis Township. As such, the number of large vehicles in April is significantly less than what is anticipated during peak operation. Data provided by the applicant for the existing and proposed facilities was used to determine the amount of daily truck traffic that will be generated by the site in Dennis Township. Based upon the data provided, the proposed Dennis Township facility could experience a peak daily truck traffic of 160 trips per day, which is divided into 80 inbound trips and 80 outbound trips.

Of the 160 total daily truck trips anticipated for the Dennis Township facility, data from the Bridgeport site indicates that approximately 21% of the truck trips will occur during the AM peak hour and 31% of the truck trips will occur during the PM peak hour. Therefore, the future maximum site traffic for the proposed Dennis Township facility is indicated in Table 2. It should be noted that based upon the operation of the existing Bridgeport facility, the peak hour traffic to be generated by the Dennis Township facility will not coincide with the peak hour of the traffic along the adjacent roadway network, specifically during peak summer roadway conditions. Typical peak summer roadway conditions occur on Friday between 4:00 PM and 6:00 PM and on Saturday between 11:00 AM and 2:00 PM. The proposed R.E. Pierson construction facility is typically not in operation during these periods and will therefore not impact the roadway network during peak summer roadway conditions.

<b>Table 2 Dennis Township Facility Trip Generation</b>							
Development	Vehicle	AM Peak Hour			PM Peak Hour		
		In	Out	Total	In	Out	Total
Construction Facility	Large Vehicles	17	17	34	25	25	50
	Passenger Vehicles	15	3	18	3	3	6
	<b>Totals</b>	<b>32</b>	<b>20</b>	<b>52</b>	<b>28</b>	<b>28</b>	<b>56</b>

The site traffic generated by the Dennis Township facility must then be distributed to the adjacent roadway network based on the routes in which the employees/patrons are expected to



travel. Conversations with the applicant indicate that approximately 60% of the large vehicle traffic will travel to/from the west, with the remaining 40% traveling to/from the east along Woodbine-Ocean View Road. As such, the large vehicle trips associated with the construction facility were distributed to the adjacent roadway network utilizing these percentages while the existing distribution of traffic along the adjacent roadway network (Figure 4) was used to determine the movement of the passenger vehicles. Figure 5 indicates the total site traffic for the proposed R.E. Pierson facility in Dennis Township. Adding the site traffic to the 2009 No-Build volumes, results in the Build volumes, which are illustrated in Figure 6.

### **Operational Analysis**

In order to measure the quality of the traffic flow for the adjacent roadways and intersections, capacity analyses for the study intersections have been completed based upon the methods outlined in the *Highway Capacity Manual (HCM 2000 Edition)*. Capacity analysis is a procedure used to estimate the ability of the roadway network to carry traffic. Capacity analyses are performed based on a level of service methodology. Level of service (LOS) is a qualitative measure that characterizes the operational conditions of a roadway or intersection based on the perceptions by motorists and passengers. Levels of service are defined for each type of facility (i.e. freeways, highways, signalized intersections, unsignalized intersections). These levels of service range from LOS A to LOS F, with a LOS A representing the best operating conditions and a LOS F representing the worst operating conditions.

The levels of service for a signalized intersection are classified in terms of delay, which is based on the extent of driver discomfort and frustration, fuel consumption and lost travel time. The delay experienced by a motorist consists of many factors that relate to control, geometrics and traffic. Some of these factors include the quality of progression, traffic signal cycle length, the green time ratio and the volume to capacity ratio. The level of service for an unsignalized intersection is determined based on the average control delay associated with each minor movement (i.e. yielding left-turn movements from the major roads and stop-controlled movements from the minor approaches). The Level of Service criteria for unsignalized and signalized intersections are provided in Table 3.

<b>Table 3 Level of Service Criteria</b>		
<b>Level of Service</b>	<b>Unsignalized Delay (sec)</b>	<b>Signalized Delay (sec)</b>
A	≤10	≤10
B	>10 and ≤15	>10 and ≤20
C	>15 and ≤25	>20 and ≤35
D	>25 and ≤35	>35 and ≤55
E	>35 and ≤50	>55 and ≤80
F	>50	>80

In order to assess the traffic impact of the proposed development, the study intersection was evaluated under the Existing, No-Build, and Build scenarios. A detailed description of the study intersection's operations under the three scenarios and a comparison summary for each intersection is provided below. The Existing, No-Build, and Build levels of service are indicated



in Figures 7, 8, and 9, respectively. As previously noted, all analyses were conducted using the seasonally adjusted traffic volumes.

#### ***Woodbine-Ocean View Road (CR 550) and Corsons Tavern Road (CR 628)***

Under existing conditions, the Woodbine-Ocean View Road/Corsons Tavern Road signalized intersection functions at an overall LOS B during both the AM and PM peak hours. All individual approaches currently operate at a LOS B or better during both peak hours.

In the 2009 No-Build and Build scenarios, all individual approaches will continue to function at a LOS B or better during both the AM and PM peak hours. Overall, the Woodbine-Ocean View Road/Corsons Tavern Road signalized intersection will continue to function at a LOS B during both peak hours. The traffic resulting from the proposed R.E. Pierson construction facility will account for approximately 3.4% and 2.2% of the total 2009 Build traffic volumes at the above-mentioned intersection during the AM and PM peak hours, respectively.

#### ***Woodbine-Ocean View Road (CR 550) and Site Driveway Intersections***

Under existing conditions, the southbound site driveway stop-controlled approaches operate at a LOS B during both the AM and PM peak hours, while the eastbound Woodbine-Ocean View Road conflicting left-turn movements currently function at a LOS A during both peak hours.

In the 2009 No-Build scenario, all stop-controlled and conflicting left-turn movements at the Woodbine-Ocean View Road/Site Driveway intersections will continue to operate at existing levels of service during both the AM and PM peak hours.

Under the 2009 Build conditions, the southbound site driveway stop-controlled approaches will function at a LOS C during both the AM and PM peak hours, with the exception of the west driveway approach, which will operate at a LOS B during the AM peak hour. Maximum queuing for the southbound site driveway approaches and eastbound Woodbine-Ocean View Road conflicting left-turn movements will be one (1) vehicle during both the AM and PM peak hours.

As previously noted, the AM and PM peak hour operation of the proposed R.E. Pierson construction facility will be staggered when compared to the peak hour of the adjacent roadway network and therefore the traffic to be generated by the proposed development will not significantly increase the peak hour volumes along Woodbine-Ocean View Road.

#### **Conclusion**

The amount of traffic to be generated by the proposed R.E. Pierson construction facility in Dennis Township will be similar to the existing Bridgeport facility. As such, it is anticipated that the proposed Dennis Township facility will generate a maximum of 160 daily truck trips during peak operation. Of these daily truck trips, approximately 21% will occur during the AM peak hour and 31% will occur during the PM peak hour. With the additional traffic for the employees and patrons, it is estimated that the proposed R.E. Pierson construction facility in Dennis Township will generate a total of 52 trips during the AM peak hour and 56 trips during the PM peak hour.



The traffic to be generated by the proposed R.E. Pierson construction facility will cause no significant changes in the levels of service at the Woodbine-Ocean View Road/Corsons Tavern Road signalized intersection during both the AM and PM peak hours. All individual approaches will operate at a LOS B or better during both peak hours, while overall the intersection will function at a LOS B during the AM and PM peak hours.

All outbound stop-controlled movements from the site driveways and inbound conflicting left-turn movements into the site driveways will operate at a LOS C or better during the AM and PM peak hours in the 2009 Build scenario. Maximum queuing for all movements will be one (1) vehicle during both peak hours.

Based upon the operation of the existing Bridgeport facility, the peak hour traffic to be generated by the Dennis Township facility will not coincide with the peak hour of the traffic along the adjacent roadway network, specifically during peak summer roadway conditions. Typical peak summer roadway conditions occur on Friday between 4:00 PM and 6:00 PM and on Saturday between 11:00 AM and 2:00 PM. As previously stated, peak truck traffic at the existing facility occurred on a typical weekday at 8:00 AM and 1:00 PM.

Please call us if you have any questions or need additional information.

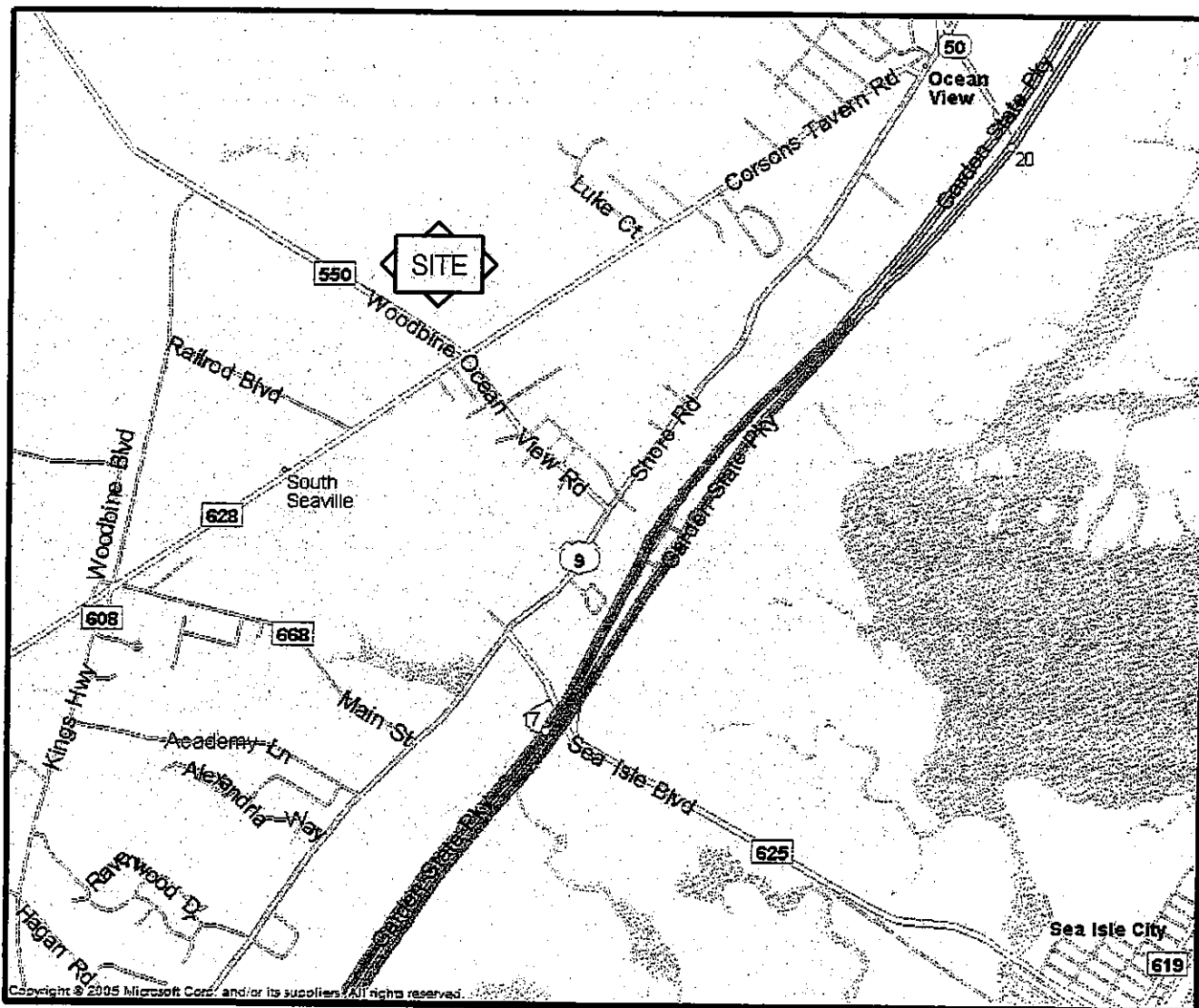
Sincerely,  
**Shropshire Associates LLC**

David R. Shropshire, P.E.  
President  
DRS/mas  
Attachments

Nathan B. Mosley  
Traffic Consultant

cc: James Pickering, Esq.  
John J. Helbig  
Curt Mitchell

(via UPS w/ attachments)  
(via UPS w/ attachments)  
(via UPS w/ attachments)



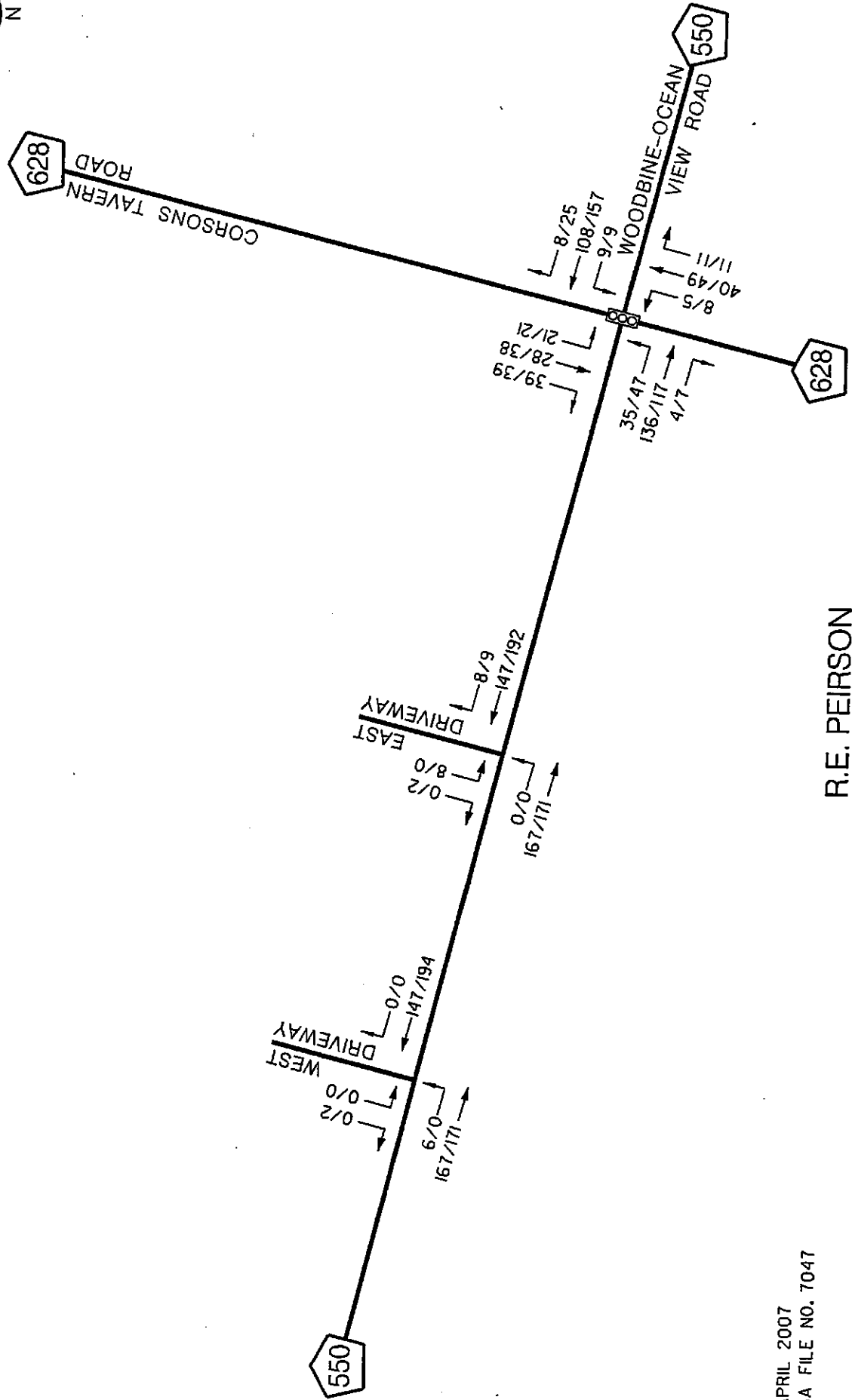
APRIL 2007  
SA FILE NO. 7047

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R.E. PIERSON  
DENNIS TOWNSHIP, CAPE MAY COUNTY, NJ



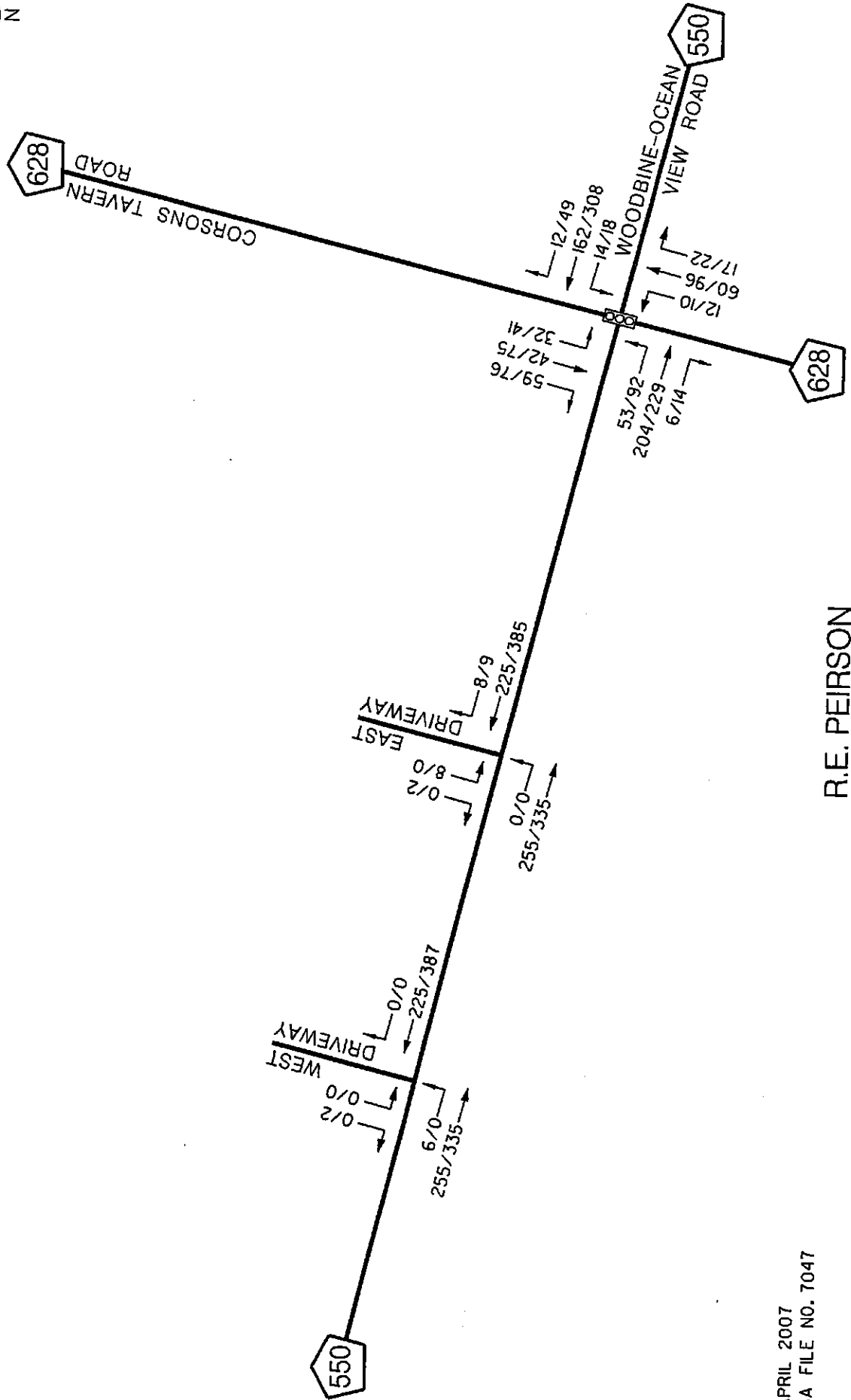
FIGURE 2A  
 EXISTING VOLUMES



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 DENNIS TOWNSHIP, CAPE MAY COUNTY, NJ

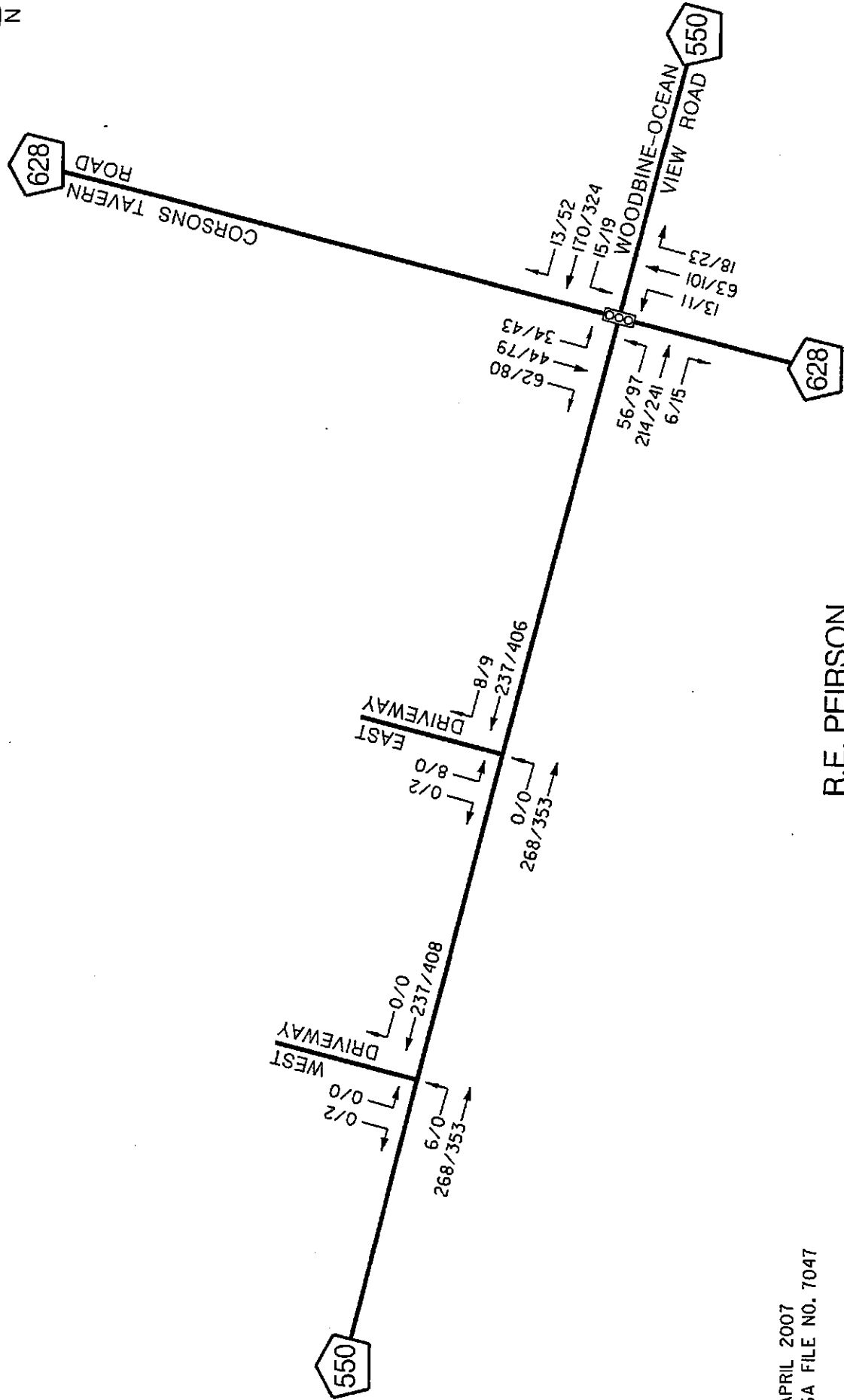
FIGURE 2B  
 SUMMER VOLUMES.



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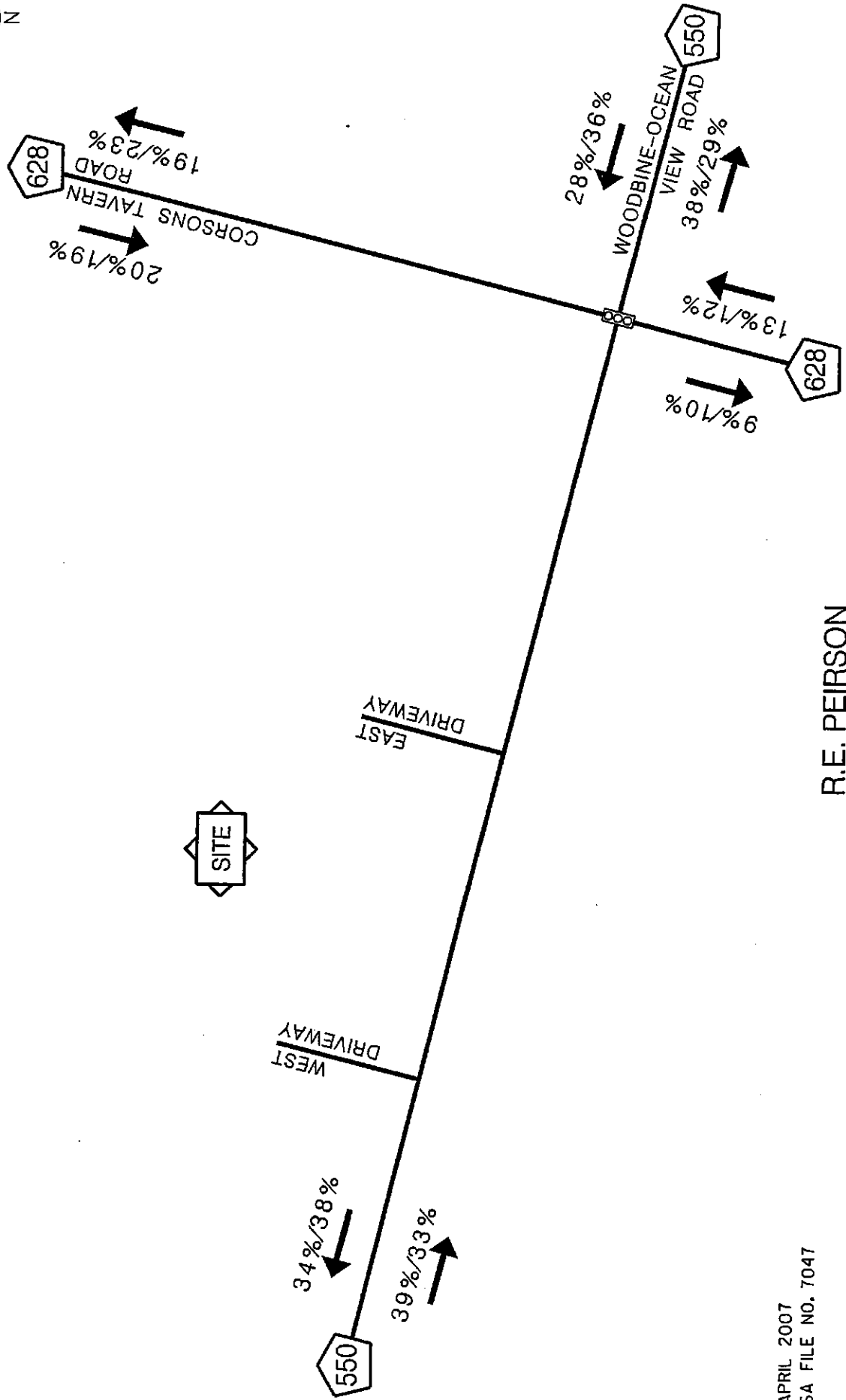
FIGURE 3  
 NO-BUILD VOLUMES



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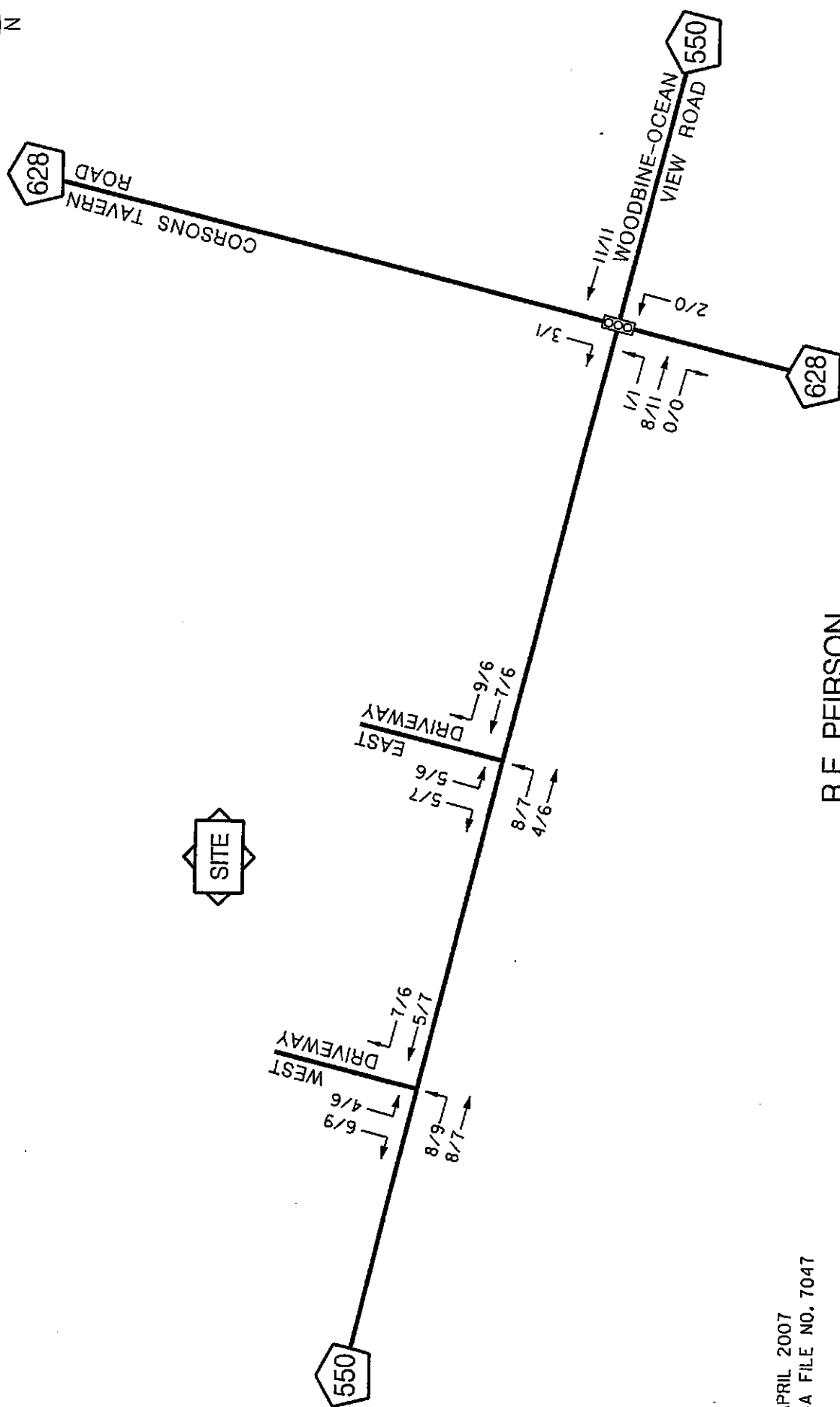
R.E. PEIRSON  
 DENNIS TOWNSHIP, CAPE MAY COUNTY, NJ

FIGURE 4  
 TRIP DISTRIBUTION



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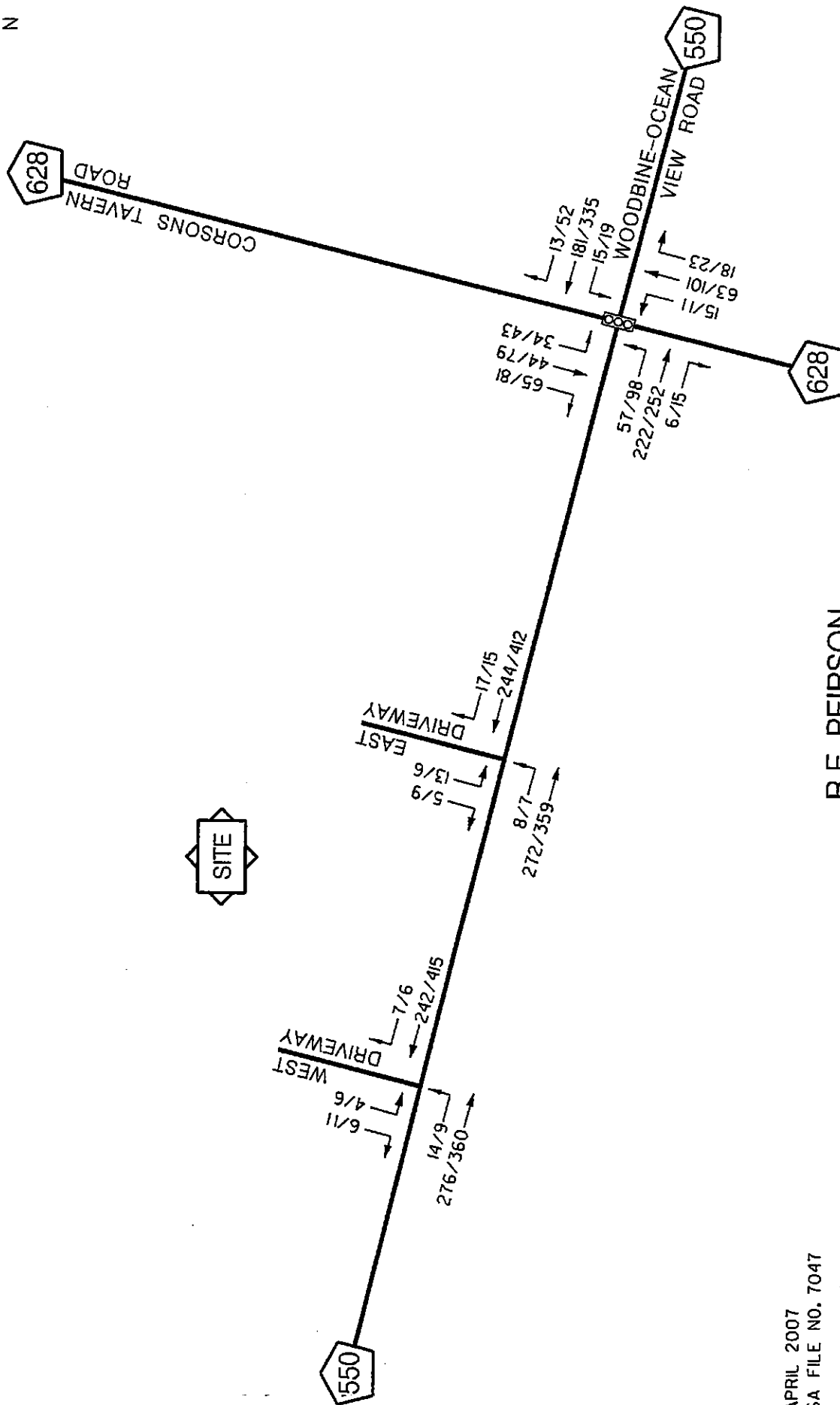
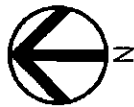
R.E. PEIRSON  
 DENNIS TOWNSHIP, CAPE MAY COUNTY, NJ



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R.E. PEIRSON  
DENNIS TOWNSHIP, CAPE MAY COUNTY, NJ

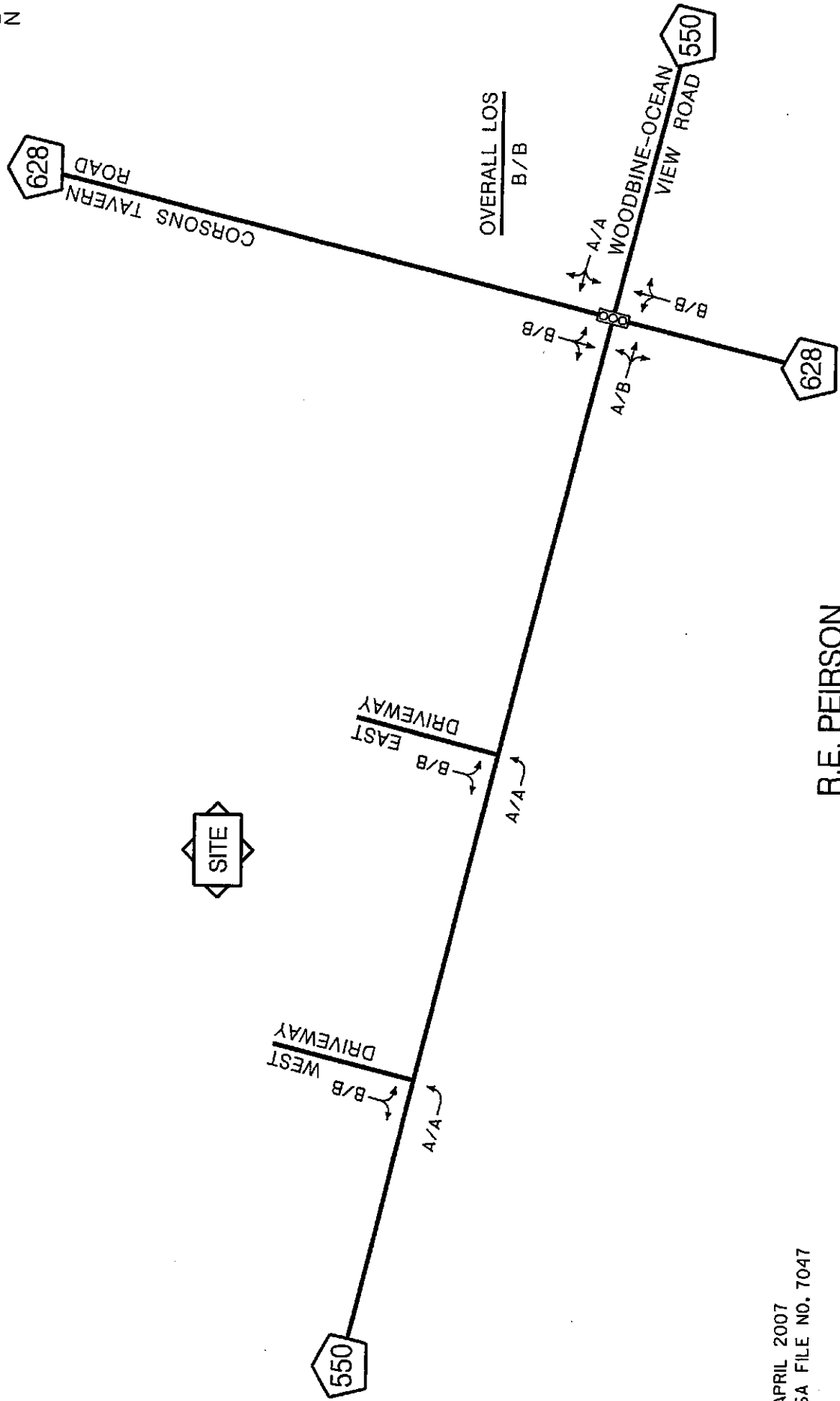
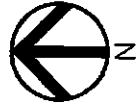
**FIGURE 6**  
**BUILD VOLUMES**



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**R.E. PEIRSON**  
 DENNIS TOWNSHIP, CAPE MAY COUNTY, NJ

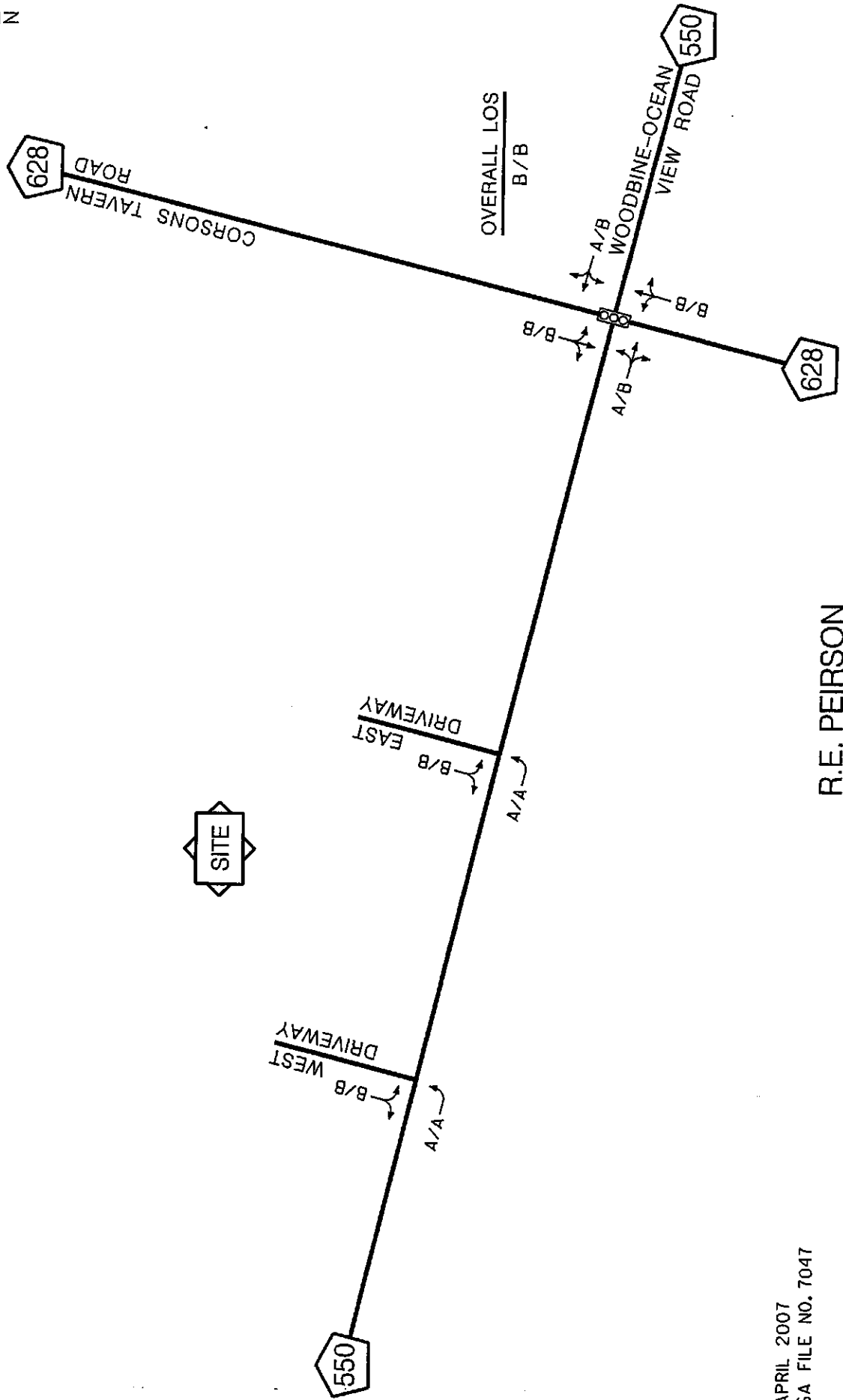
FIGURE 7  
EXISTING LEVELS OF SERVICE



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DENNIS TOWNSHIP, CAPE MAY COUNTY, NJ

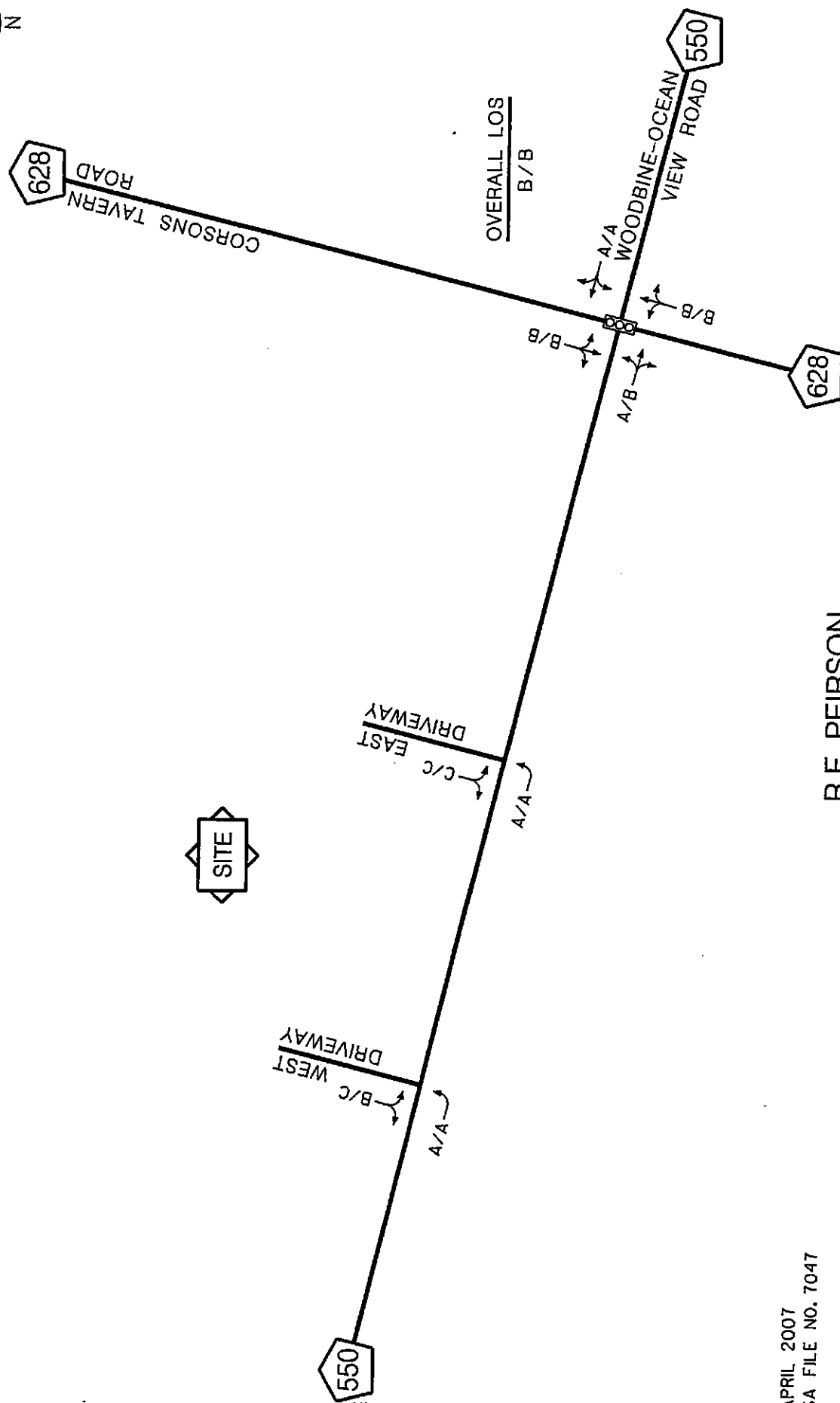
FIGURE 8  
 NO-BUILD LEVELS OF SERVICE



APRIL 2007  
 SA FILE NO. 7047

R.E. PEIRSON  
 DENNIS TOWNSHIP, CAPE MAY COUNTY, NJ





R.E. PEIRSON  
DENNIS TOWNSHIP, CAPE MAY COUNTY, NJ

# Shropshire Associates LLC

662 South Main Street  
Lumberton, New Jersey 08048

N/S Route: Corson Tavern Road  
E/W Route: Woodbine-Ocean View Rd.  
Dennis Twp./Cape May Co.NJ  
Clear/Tues./JA/2584

File Name : 70470002  
Site Code : 70470002  
Start Date : 4/3/2007  
Page No : 1

## Groups Printed - Unshifted - Heavy Vehicles

	Corson Tavern Road Southbound			Woodbine-Ocean View Road Westbound			Corson Tavern Road Northbound			Woodbine-Ocean View Road Eastbound			Int. Total
Start Time	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	Right	Thru	Left	
06:00 AM	0	1	1	0	4	0	0	2	0	0	5	0	13
06:15 AM	3	0	1	1	7	1	0	4	1	0	15	2	35
06:30 AM	4	3	2	0	17	0	0	10	0	0	23	4	63
06:45 AM	10	4	5	3	20	1	2	7	1	0	24	3	80
Total	17	8	9	4	48	2	2	23	2	0	67	9	191
07:00 AM	3	7	6	2	20	0	1	3	2	1	18	7	70
07:15 AM	11	2	3	3	24	2	0	2	3	4	31	4	89
07:30 AM	11	10	8	2	30	3	2	4	1	2	39	10	122
07:45 AM	18	10	5	4	36	2	2	16	6	1	39	12	151
Total	43	29	22	11	110	7	5	25	12	8	127	33	432
08:00 AM	5	4	6	0	22	2	1	5	0	1	29	6	81
08:15 AM	5	4	2	2	20	2	6	15	1	0	29	7	93
08:30 AM	6	9	10	5	28	2	3	9	1	0	39	6	118
08:45 AM	10	5	1	5	30	2	5	9	0	0	21	4	92
Total	26	22	19	12	100	8	15	38	2	1	118	23	384
01:00 PM	14	6	3	5	34	1	1	9	0	2	28	8	111
01:15 PM	15	10	4	10	30	3	1	7	1	3	24	6	114
01:30 PM	9	9	3	6	31	2	4	11	1	1	36	8	121
01:45 PM	8	10	6	4	38	0	1	7	1	1	27	6	109
Total	46	35	16	25	133	6	7	34	3	7	115	28	455
02:00 PM	8	6	0	3	34	3	2	8	0	1	29	7	101
02:15 PM	8	14	4	8	33	1	2	7	1	0	29	12	119
02:30 PM	10	8	5	6	24	4	3	16	3	0	27	7	113
02:45 PM	10	12	3	8	28	1	3	9	0	4	23	8	109
Total	36	40	12	25	119	9	10	40	4	5	108	34	442
03:00 PM	8	11	5	5	36	2	6	7	0	2	26	14	122
03:15 PM	14	9	3	6	36	1	1	12	3	2	27	12	126
03:30 PM	11	10	6	5	46	2	3	11	0	2	37	14	147
03:45 PM	6	8	7	9	39	4	1	19	2	1	27	7	130
Total	39	38	21	25	157	9	11	49	5	7	117	47	525
Grand Total	207	172	99	102	667	41	50	209	28	28	652	174	2429
Apprch %	43.3	36	20.7	12.6	82.3	5.1	17.4	72.8	9.8	3.3	76.3	20.4	
Total %	8.5	7.1	4.1	4.2	27.5	1.7	2.1	8.6	1.2	1.2	26.8	7.2	
Unshifted	191	171	93	97	539	41	48	205	28	24	542	161	2140
% Unshifted	92.3	99.4	93.9	95.1	80.8	100	96	98.1	100	85.7	83.1	92.5	88.1
Heavy Vehicles	16	1	6	5	128	0	2	4	0	4	110	13	289
% Heavy Vehicles	7.7	0.6	6.1	4.9	19.2	0	4	1.9	0	14.3	16.9	7.5	11.9

# Shropshire Associates LLC

662 South Main Street  
Lumberton, New Jersey 08048

N/S Route: Corson Tavern Road  
E/W Route: Woodbine-Ocean View Rd.  
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File Name : 70470002  
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Start Date : 4/3/2007  
Page No : 2

	Corson Tavern Road Southbound				Woodbine-Ocean View Road Westbound				Corson Tavern Road Northbound				Woodbine-Ocean View Road Eastbound				
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 06:00 AM to 08:45 AM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 07:30 AM																	
07:30 AM	11	10	8	29	2	30	3	35	2	4	1	7	2	39	10	51	122
07:45 AM	18	10	5	33	4	36	2	42	2	16	6	24	1	39	12	52	151
08:00 AM	5	4	6	15	0	22	2	24	1	5	0	6	1	29	6	36	81
08:15 AM	5	4	2	11	2	20	2	24	6	15	1	22	0	29	7	36	93
Total Volume	39	28	21	88	8	108	9	125	11	40	8	59	4	136	35	175	447
% App. Total	44.3	31.8	23.9		6.4	86.4	7.2		18.6	67.8	13.6		2.3	77.7	20		
PHF	.542	.700	.656	.667	.500	.750	.750	.744	.458	.625	.333	.615	.500	.872	.729	.841	.740
Peak Hour Analysis From 01:00 PM to 03:45 PM - Peak 1 of 1																	
Peak Hour for Entire Intersection Begins at 03:00 PM																	
03:00 PM	8	11	5	24	5	36	2	43	6	7	0	13	2	26	14	42	122
03:15 PM	14	9	3	26	6	36	1	43	1	12	3	16	2	27	12	41	126
03:30 PM	11	10	6	27	5	46	2	53	3	11	0	14	2	37	14	53	147
03:45 PM	6	8	7	21	9	39	4	52	1	19	2	22	1	27	7	35	130
Total Volume	39	38	21	98	25	157	9	191	11	49	5	65	7	117	47	171	525
% App. Total	39.8	38.8	21.4		13.1	82.2	4.7		16.9	75.4	7.7		4.1	68.4	27.5		
PHF	.696	.864	.750	.907	.694	.853	.563	.901	.458	.645	.417	.739	.875	.791	.839	.807	.893

# Shropshire Associates LLC

662 South Main Street  
Lumberton, New Jersey 08048

N/S Route: R/E. Pierson East Access  
E/W Route: Woodbine-Ocean View Rd.  
Dennis Twp./Cape May Co. NJ  
Clear/Tues./BEB/2538

File Name : 70470003  
Site Code : 70470003  
Start Date : 4/3/2007  
Page No : 1

## Groups Printed- Unshifted - East Access Heavy Vehicles

Start Time	R.E Pierson East Access Southbound		Woodbine-Ocean View Road Westbound		Woodbine-Ocean View Road Eastbound		Int. Total
	Right	Left	Right	Thru	Thru	Left	
06:00 AM	0	1	0	7	12	0	20
06:15 AM	4	8	0	11	8	0	31
06:30 AM	0	0	0	22	27	0	49
06:45 AM	0	1	0	22	29	0	52
Total	4	10	0	62	76	0	152
07:00 AM	0	1	1	22	22	0	46
07:15 AM	0	1	1	36	43	0	81
07:30 AM	0	2	2	41	48	0	93
07:45 AM	0	4	4	51	41	0	100
Total	0	8	8	150	154	0	320
08:00 AM	0	1	1	33	51	0	86
08:15 AM	0	2	1	24	32	0	59
08:30 AM	0	0	0	38	38	0	76
08:45 AM	0	1	2	36	28	0	67
Total	0	4	4	131	149	0	288
01:00 PM	0	1	3	44	30	0	78
01:15 PM	0	4	5	39	27	0	75
01:30 PM	0	4	2	37	37	1	81
01:45 PM	0	1	0	41	31	0	73
Total	0	10	10	161	125	1	307
02:00 PM	0	2	2	37	30	0	71
02:15 PM	0	0	5	35	34	0	74
02:30 PM	2	1	1	35	33	0	72
02:45 PM	0	0	5	32	32	1	70
Total	2	3	13	139	129	1	287
03:00 PM	1	0	2	39	41	0	83
03:15 PM	1	0	1	52	33	0	87
03:30 PM	0	0	4	49	52	0	105
03:45 PM	0	0	2	45	37	0	84
Total	2	0	9	185	163	0	359
Grand Total	8	35	44	828	796	2	1713
Apprch %	18.6	81.4	5	95	99.7	0.3	
Total %	0.5	2	2.6	48.3	46.5	0.1	
Unshifted	0	1	0	740	707	1	1449
% Unshifted	0	2.9	0	89.4	88.8	50	84.6
East Access Heavy Vehicles	8	34	44	88	89	1	264
% East Access Heavy Vehicles	100	97.1	100	10.6	11.2	50	15.4

	R.E Pierson East Access Southbound			Woodbine-Ocean View Road Westbound			Woodbine-Ocean View Road Eastbound			
Start Time	Right	Left	App. Total	Right	Thru	App. Total	Thru	Left	App. Total	Int. Total
Peak Hour Analysis From 06:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 07:15 AM										
07:15 AM	0	1	1	1	36	37	43	0	43	81
07:30 AM	0	2	2	2	41	43	48	0	48	93
07:45 AM	0	4	4	4	51	55	41	0	41	100
08:00 AM	0	1	1	1	33	34	51	0	51	86
Total Volume	0	8	8	8	161	169	183	0	183	360
% App. Total	0	100		4.7	95.3		100	0		
PHF	.000	.500	.500	.500	.789	.768	.897	.000	.897	.900

Peak Hour Analysis From 01:00 PM to 03:45 PM - Peak 1 of 1  
Peak Hour for Entire Intersection Begins at 03:00 PM

03:00 PM	1	0	1	2	39	41	41	0	41	83
03:15 PM	1	0	1	1	52	53	33	0	33	87
03:30 PM	0	0	0	4	49	53	52	0	52	105
03:45 PM	0	0	0	2	45	47	37	0	37	84
Total Volume	2	0	2	9	185	194	163	0	163	359
% App. Total	100	0		4.6	95.4		100	0		
PHF	.500	.000	.500	.563	.889	.915	.784	.000	.784	.855



# Shropshire Associates LLC

662 South Main Street  
Lumberton, New Jersey 08048

N/S Route: Oak Grove Road (CR 671)  
E/W Route: R.E. Pierson Access  
Logan Twp./Gloucester Co. NJ  
Clear/Tues./CA/2585

File Name : 70470001  
Site Code : 70470001  
Start Date : 4/3/2007  
Page No : 1

## Groups Printed- Unshifted

Start Time	Oak Grove Road Southbound		Oak Grove Road Northbound		R.E. Pierson Access Eastbound		Int. Total
	Right	Thru	Thru	Left	Right	Left	
06:00 AM	0	3	2	0	0	0	5
06:15 AM	1	3	2	0	0	0	6
06:30 AM	2	2	2	3	1	0	10
06:45 AM	1	3	7	1	0	0	12
Total	4	11	13	4	1	0	33
07:00 AM	1	4	9	2	1	1	18
07:15 AM	3	5	7	2	0	0	17
07:30 AM	1	1	5	2	0	0	9
07:45 AM	2	2	5	0	1	1	11
Total	7	12	26	6	2	2	55
08:00 AM	1	3	2	0	1	1	8
08:15 AM	0	3	3	0	0	0	6
08:30 AM	1	2	3	1	0	1	8
08:45 AM	0	3	0	0	0	1	4
Total	2	11	8	1	1	3	26
01:00 PM	1	4	4	0	0	0	9
01:15 PM	0	5	2	0	0	1	8
01:30 PM	3	4	4	3	0	0	14
01:45 PM	1	5	5	0	1	1	13
Total	5	18	15	3	1	2	44
02:00 PM	1	5	4	0	3	3	16
02:15 PM	0	4	5	0	1	0	10
02:30 PM	1	0	5	0	0	0	6
02:45 PM	0	4	3	1	1	2	11
Total	2	13	17	1	5	5	43
03:00 PM	2	1	4	0	0	2	9
03:15 PM	0	9	4	0	0	0	13
03:30 PM	0	9	5	1	1	0	16
03:45 PM	0	8	8	0	0	0	16
Total	2	27	21	1	1	2	54
Grand Total	22	92	100	16	11	14	255
Apprch %	19.3	80.7	86.2	13.8	44	56	
Total %	8.6	36.1	39.2	6.3	4.3	5.5	

	Oak Grove Road Southbound			Oak Grove Road Northbound			R.E. Pierson Access Eastbound			
Start Time	Right	Thru	App. Total	Thru	Left	App. Total	Right	Left	App. Total	Int. Total
Peak Hour Analysis From 06:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 06:30 AM										
06:30 AM	2	2	4	2	3	5	1	0	1	10
06:45 AM	1	3	4	7	1	8	0	0	0	12
07:00 AM	1	4	5	9	2	11	1	1	2	18
07:15 AM	3	5	8	7	2	9	0	0	0	17
Total Volume	7	14	21	25	8	33	2	1	3	57
% App. Total	33.3	66.7		75.8	24.2		66.7	33.3		
PHF	.583	.700	.656	.694	.667	.750	.500	.250	.375	.792

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Logan Twp./Gloucester Co. NJ

Clear/Tues:/CA/2585

File Name : 70470001

Site Code : 70470001

Start Date : 4/3/2007

Page No : 2

	Oak Grove Road Southbound			Oak Grove Road Northbound			R.E. Pierson Access Eastbound			
Start Time	Right	Thru	App. Total	Thru	Left	App. Total	Right	Left	App. Total	Int. Total
Peak Hour Analysis From 01:00 PM to 03:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 03:00 PM										
03:00 PM	2	1	3	4	0	4	0	2	2	9
03:15 PM	0	9	9	4	0	4	0	0	0	13
03:30 PM	0	9	9	5	1	6	1	0	1	16
03:45 PM	0	8	8	8	0	8	0	0	0	16
Total Volume	2	27	29	21	1	22	1	2	3	54
% App. Total	6.9	93.1		95.5	4.5		33.3	66.7		
PHF	.250	.750	.806	.656	.250	.688	.250	.250	.375	.844



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Clear/Tues./CA/2585

File Name : 70470001  
Site Code : 70470001  
Start Date : 4/3/2007  
Page No : 1

## Groups Printed- Heavy Vehicles

Start Time	Oak Grove Road Southbound		Oak Grove Road Northbound		R.E. Pierson Access Eastbound		Int. Total
	Right	Thru	Thru	Left	Right	Left	
06:30 AM	1	0	0	2	0	0	3
06:45 AM	4	0	1	2	1	1	9
Total	5	0	1	4	1	1	12
07:00 AM	1	0	0	0	1	5	7
07:15 AM	2	1	1	0	0	1	5
07:30 AM	0	0	0	0	2	0	2
07:45 AM	2	0	0	1	0	0	3
Total	5	1	1	1	3	6	17
08:00 AM	1	0	0	0	1	2	4
08:15 AM	2	0	0	2	1	1	6
08:30 AM	3	0	0	2	2	1	8
08:45 AM	0	1	0	2	3	3	9
Total	6	1	0	6	7	7	27
01:00 PM	5	0	0	2	2	3	12
01:15 PM	2	0	0	2	2	2	8
01:30 PM	3	0	0	1	4	2	10
01:45 PM	3	0	0	2	3	1	9
Total	13	0	0	7	11	8	39
02:00 PM	1	0	0	2	1	2	6
02:15 PM	0	0	0	0	0	2	2
02:30 PM	2	0	0	1	2	1	6
02:45 PM	0	0	0	0	2	1	3
Total	3	0	0	3	5	6	17
03:00 PM	0	0	0	2	0	0	2
03:15 PM	0	0	0	0	2	0	2
Total	0	0	0	2	2	0	4
Grand Total	32	2	2	23	29	28	116
Apprch %	94.1	5.9	8	92	50.9	49.1	
Total %	27.6	1.7	1.7	19.8	25	24.1	

	Oak Grove Road Southbound			Oak Grove Road Northbound			R.E. Pierson Access Eastbound			
Start Time	Right	Thru	App. Total	Thru	Left	App. Total	Right	Left	App. Total	Int. Total
Peak Hour Analysis From 06:00 AM to 08:45 AM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 08:00 AM										
08:00 AM	1	0	1	0	0	0	1	2	3	4
08:15 AM	2	0	2	0	2	2	1	1	2	6
08:30 AM	3	0	3	0	2	2	2	1	3	8
08:45 AM	0	1	1	0	2	2	3	3	6	9
Total Volume	6	1	7	0	6	6	7	7	14	27
% App. Total	85.7	14.3		0	100		50	50		
PHF	.500	.250	.583	.000	.750	.750	.583	.583	.583	.750

# Shropshire Associates LLC

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Lumberton, New Jersey 08048

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E/W Route: R.E. Pierson Access

Logan Twp./Gloucester Co. NJ

Clear/Tues./CA/2585

File Name : 70470001

Site Code : 70470001

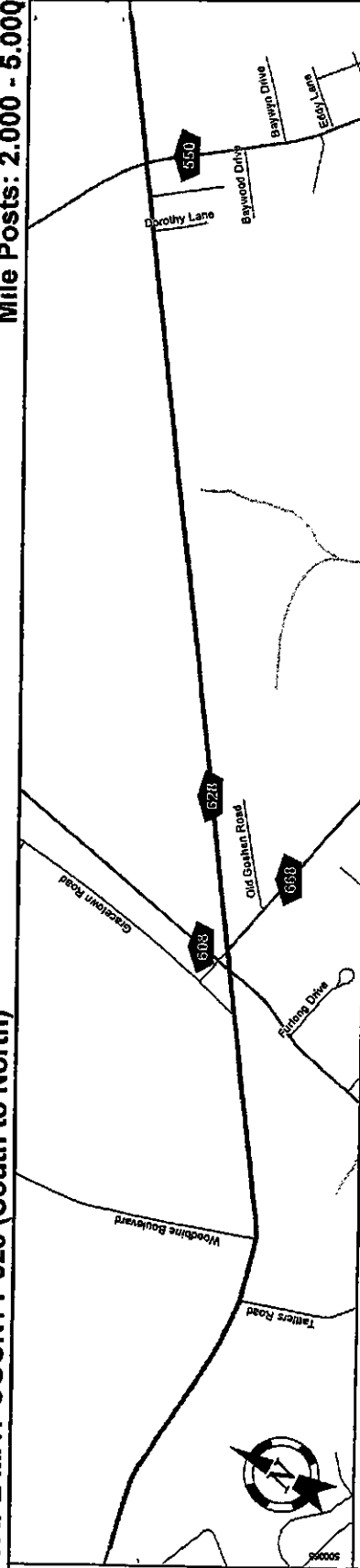
Start Date : 4/3/2007

Page No : 2

	Oak Grove Road Southbound			Oak Grove Road Northbound			R.E. Pierson Access Eastbound			
Start Time	Right	Thru	App. Total	Thru	Left	App. Total	Right	Left	App. Total	Int. Total
Peak Hour Analysis From 01:00 PM to 03:45 PM - Peak 1 of 1										
Peak Hour for Entire Intersection Begins at 01:00 PM										
01:00 PM	5	0	5	0	2	2	2	3	5	12
01:15 PM	2	0	2	0	2	2	2	2	4	8
01:30 PM	3	0	3	0	1	1	4	2	6	10
01:45 PM	3	0	3	0	2	2	3	1	4	9
Total Volume	13	0	13	0	7	7	11	8	19	39
% App. Total	100	0		0	100		57.9	42.1		
PHF	.650	.000	.650	.000	.875	.875	.688	.667	.792	.813

# CAPE MAY COUNTY 628 (South to North)

Mile Posts: 2.000 - 5.000

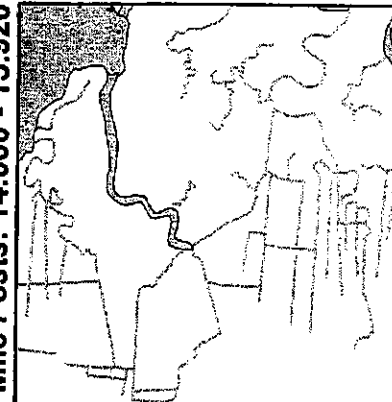


Secondary Direction		Primary Direction	
Interstate Route	287	Interstate Route	287
US Route	22	US Route	22
NJ Route	21	NJ Route	21
County Road	689	County Road	689
Interchange Number	2	Interchange Number	2
Grade		Grade	
Separated Interchange		Separated Interchange	
Traffic Signal		Traffic Signal	
Toll Plaza		Toll Plaza	
Traffic Monitoring Sites		Traffic Monitoring Sites	
Road Underpass		Road Underpass	
Road Overpass		Road Overpass	
Street Name	Dennisville Road	Street Name	Dennisville Road
Jurisdiction	County	Jurisdiction	County
Functional Class	Urban Collector	Functional Class	Urban Collector
Federal Aid Sys	STP	Federal Aid Sys	STP
Speed Limit	50	Speed Limit	50
Number of Lanes	40	Number of Lanes	40
Med. Type	2	Med. Type	2
Med. Width	None	Med. Width	None
Pavement	0	Pavement	0
Shoulder	21	Shoulder	21
Traffic Volume	4	Traffic Volume	4
Traffic Sta. ID	2	Traffic Sta. ID	2
Structure No.	7	Structure No.	7
Enlarged View	23	Enlarged View	23

SRI = 05000628

Date last inventoried: August 1999

**Mile Posts: 14.000 - 15.920**



Secondary direction milepost	Units in miles
	17.0

**SRI = 00000550**

**SRI = 00000550**

FEBRUARY 7, 2003

WOODBINE-OCEANVIEW ROAD (CR 550) AND  
CORSONS TAVERN ROAD (CR 628)  
Township of Dennis, Cape May County, NJ  
TRAFFIC SIGNAL TIMINGS

VARIABLE CYCLE LENGTH -- 41 TO 64 SECONDS

PHASE	<u>1,2,3,4,5</u>	<u>6,7,8,9</u>	TIME (SEC.)
1. Woodbine-Oceanview Road (CR 550) ROW	G	R	15-30
Change	Y	R	5
Clearance	R	R	2
2. Corsons Tavern Road (CR 628) ROW	R	G	12-20
Change	R	Y	5
Clearance	R	R	2
Emergency Flash	Y	R	50-60 per min.

MANUAL CONTROL TO BE DISCONNECTED. MEMORY CIRCUITS TO BE 'OFF'.

VEHICLE INTERVAL TO BE SET AT 3 SECONDS.

IF NO ACTUATION OCCURS, SIGNAL SHALL REST IN ALL-RED INTERVAL.

IF ACTUATION OF A PHASE OCCURS DURING THE CHANGE (YELLOW) OR  
CLEARANCE (ALL-RED) INTERVALS OF THE SAME PHASE, THE CHANGE  
AND CLEARANCE INTERVALS SHALL TIME OUT FULLY BEFORE REVERTING  
TO THE ROW (GREEN) PHASE.

# SHORT REPORT

General Information						Site Information					
Analyst	nbm ea1					Intersection	Woodbine-Ocean/Corsons Tavern				
Agency or Co.	Shropshire Associates LLC					Area Type	All other areas				
Date Performed	4/13/2007					Jurisdiction	Cape May County				
Time Period	Existing AM Peak Hour					Analysis Year	2007				

## Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Lane Group		LTR			LTR			LTR			LTR	
Volume (vph)	53	204	6	14	162	12	12	60	17	32	42	59
% Heavy Vehicles	2	2	2	2	2	2	2	2	2	2	2	2
PHF	0.84	0.84	0.84	0.74	0.74	0.74	0.62	0.62	0.62	0.67	0.67	0.67
Pretimed/Actuated (P/A)	A	A	A	A	A	A	A	A	A	A	A	A
Startup Lost Time		2.0			2.0			2.0			2.0	
Extension of Effective Green		5.0			5.0			5.0			5.0	
Arrival Type		3			3			3			3	
Unit Extension		2.0			2.0			2.0			2.0	
Ped/Bike/RTOR Volume	0	0	1	0	0	1	0	0	2	0	0	6
Lane Width		12.0			12.0			12.0			12.0	
Parking/Grade/Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking/Hour												
Bus Stops/Hour		0			0			0			0	
Minimum Pedestrian Time												

Phasing	EW Perm	02	03	04	NS Perm	06	07	08
Timing	G = 30.0	G =	G =	G =	G = 20.0	G =	G =	G =
	Y = 7	Y =	Y =	Y =	Y = 7	Y =	Y =	Y =
Duration of Analysis (hrs) = 0.25						Cycle Length C = 64.0		

## Lane Group Capacity, Control Delay, and LOS Determination

	EB		WB			NB			SB		
Adjusted Flow Rate		312			253			140			190
Lane Group Capacity		857			920			620			568
v/c Ratio		0.36			0.28			0.23			0.33
Green Ratio		0.52			0.52			0.36			0.36
Uniform Delay $d_1$		9.2			8.7			14.3			14.9
Delay Factor k		0.04			0.04			0.04			0.04
Incremental Delay $d_2$		0.1			0.1			0.1			0.1
PF Factor		1.000			1.000			1.000			1.000
Control Delay		9.3			8.8			14.4			15.1
Lane Group LOS		A			A			B			B
Approach Delay	9.3		8.8			14.4			15.1		
Approach LOS	A		A			B			B		
Intersection Delay	11.2		Intersection LOS						B		

# SHORT REPORT

## General Information

Analyst *nbm ep1*  
 Agency or Co. *Shropshire Associates LLC*  
 Date Performed *4/13/2007*  
 Time Period *Existing PM Peak Hour*

## Site Information

Intersection *Woodbine-Ocean/Corsons Tavern*  
 Area Type *All other areas*  
 Jurisdiction *Cape May County*  
 Analysis Year *2007*

## Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Lane Group		LTR			LTR			LTR			LTR	
Volume (vph)	92	229	14	18	308	49	10	96	22	41	75	76
% Heavy Vehicles	2	2	2	2	2	2	2	2	2	2	2	2
PHF	0.81	0.81	0.81	0.90	0.90	0.90	0.74	0.74	0.74	0.91	0.91	0.91
Pretimed/Actuated (P/A)	A	A	A	A	A	A	A	A	A	A	A	A
Startup Lost Time		2.0			2.0			2.0			2.0	
Extension of Effective Green		5.0			5.0			5.0			5.0	
Arrival Type		3			3			3			3	
Unit Extension		2.0			2.0			2.0			2.0	
Ped/Bike/RTOR Volume	0	0	1	0	0	5	0	0	2	0	0	8
Lane Width		12.0			12.0			12.0			12.0	
Parking/Grade/Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking/Hour												
Bus Stops/Hour		0			0			0			0	
Minimum Pedestrian Time												
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 20.0	G =	G =	G =				
	Y = 7	Y =	Y =	Y =	Y = 7	Y =	Y =	Y =				
Duration of Analysis (hrs) = 0.25				Cycle Length C = 64.0								

## Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
Adjusted Flow Rate		413			411			171			202	
Lane Group Capacity		770			919			635			575	
v/c Ratio		0.54			0.45			0.27			0.35	
Green Ratio		0.52			0.52			0.36			0.36	
Uniform Delay $d_1$		10.4			9.8			14.5			15.0	
Delay Factor k		0.07			0.04			0.04			0.04	
Incremental Delay $d_2$		0.4			0.1			0.1			0.1	
PF Factor		1.000			1.000			1.000			1.000	
Control Delay		10.8			9.9			14.6			15.2	
Lane Group LOS		B			A			B			B	
Approach Delay		10.8			9.9			14.6			15.2	
Approach LOS		B			A			B			B	
Intersection Delay		11.8			Intersection LOS						B	

## TWO-WAY STOP CONTROL SUMMARY

### General Information

Analyst *nbm ea2*  
 Agency/Co. *Shropshire Associates LLC*  
 Date Performed *4/13/2007*  
 Analysis Time Period *Existing AM Peak Hour*

### Site Information

Intersection *Woodbine-Ocean View/East Acces*  
 Jurisdiction *Cape May County*  
 Analysis Year *2007*

Project Description *7047 - R.E. Pierson*

East/West Street: *Woodbine-Ocean View (CR 550)*

North/South Street: *East Site Access*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	0	255			225	8
Peak-Hour Factor, PHF	0.90	0.90	1.00	1.00	0.77	0.77
Hourly Flow Rate, HFR (veh/h)	0	283	0	0	292	10
Percent Heavy Vehicles	2	—	—	0	—	—
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	LT					TR
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				8		0
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.50	1.00	0.50
Hourly Flow Rate, HFR (veh/h)	0	0	0	16	0	0
Percent Heavy Vehicles	0	0	0	2	0	2
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					LR	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	0						16	
C (m) (veh/h)	1259						477	
v/c	0.00						0.03	
95% queue length	0.00						0.10	
Control Delay (s/veh)	7.9						12.8	
LOS	A						B	
Approach Delay (s/veh)	—	—					12.8	
Approach LOS	—	—					B	



## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information	
Analyst	nbm ep2		Intersection	Woodbine-Ocean View/East Acces
Agency/Co.	Shropshire Associates LLC		Jurisdiction	Cape May County
Date Performed	4/13/2007		Analysis Year	2007
Analysis Time Period	Existing PM Peak Hour			

Project Description 7047 - R.E. Pierson

East/West Street: Woodbine-Ocean View (CR 550)

North/South Street: East Site Access

Intersection Orientation: East-West

Study Period (hrs): 0.25

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	0	335			385	9
Peak-Hour Factor, PHF	0.78	0.78	1.00	1.00	0.92	0.92
Hourly Flow Rate, HFR (veh/h)	0	429	0	0	418	9
Percent Heavy Vehicles	2	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	LT					TR
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				0		2
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.50	1.00	0.50
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	4
Percent Heavy Vehicles	0	0	0	2	0	2
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					LR	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	0						4	
C (m) (veh/h)	1132						632	
v/c	0.00						0.01	
95% queue length	0.00						0.02	
Control Delay (s/veh)	8.2						10.7	
LOS	A						B	
Approach Delay (s/veh)	--	--					10.7	
Approach LOS	--	--					B	

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information	
Analyst	nbm ea3		Intersection	Woodbine-Ocean View/West Acces
Agency/Co.	Shropshire Associates LLC		Jurisdiction	Cape May County
Date Performed	4/13/2007		Analysis Year	2007
Analysis Time Period	Existing AM Peak Hour			

Project Description 7047 - R.E. Pierson

East/West Street: Woodbine-Ocean View (CR 550)

North/South Street: West Site Access

Intersection Orientation: East-West

Study Period (hrs): 0.25

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	6	255			225	0
Peak-Hour Factor, PHF	0.90	0.90	1.00	1.00	0.77	0.77
Hourly Flow Rate, HFR (veh/h)	6	283	0	0	292	0
Percent Heavy Vehicles	2	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	LT					TR
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				1		0
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.25	1.00	0.25
Hourly Flow Rate, HFR (veh/h)	0	0	0	4	0	0
Percent Heavy Vehicles	0	0	0	2	0	2
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					LR	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	6						4	
C (m) (veh/h)	1270						470	
v/c	0.00						0.01	
95% queue length	0.01						0.03	
Control Delay (s/veh)	7.8						12.7	
LOS	A						B	
Approach Delay (s/veh)	--	--					12.7	
Approach LOS	--	--					B	

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information	
Analyst	nbm ep3		Intersection	Woodbine-Ocean View/West Acces
Agency/Co.	Shropshire Associates LLC		Jurisdiction	Cape May County
Date Performed	4/13/2007		Analysis Year	2007
Analysis Time Period	Existing PM Peak Hour			

Project Description 7047 - R.E. Pierson	
East/West Street: Woodbine-Ocean View (CR 550)	North/South Street: West Site Access
Intersection Orientation: East-West	Study Period (hrs): 0.25

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	0	335			387	0
Peak-Hour Factor, PHF	0.78	0.78	1.00	1.00	0.92	0.92
Hourly Flow Rate, HFR (veh/h)	0	429	0	0	420	0
Percent Heavy Vehicles	2	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	LT					TR
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				0		2
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.50	1.00	0.50
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	4
Percent Heavy Vehicles	0	0	0	2	0	2
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					LR	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	0						4	
C (m) (veh/h)	1139						633	
v/c	0.00						0.01	
95% queue length	0.00						0.02	
Control Delay (s/veh)	8.2						10.7	
LOS	A						B	
Approach Delay (s/veh)	--	--				10.7		
Approach LOS	--	--				B		

# SHORT REPORT

General Information						Site Information					
Analyst	nbm na1					Intersection	Woodbine-Ocean/Corsons Tavern				
Agency or Co.	Shropshire Associates LLC					Area Type	All other areas				
Date Performed	4/13/2007					Jurisdiction	Cape May County				
Time Period	No-Build AM Peak Hour					Analysis Year	2009				

Volume and Timing Input												
	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Lane Group		LTR			LTR			LTR			LTR	
Volume (vph)	56	214	6	15	170	13	13	65	18	34	44	62
% Heavy Vehicles	2	2	2	2	2	2	2	2	2	2	2	2
PHF	0.84	0.84	0.84	0.74	0.74	0.74	0.62	0.62	0.62	0.67	0.67	0.67
Pretimed/Actuated (P/A)	A	A	A	A	A	A	A	A	A	A	A	A
Startup Lost Time		2.0			2.0			2.0			2.0	
Extension of Effective Green		5.0			5.0			5.0			5.0	
Arrival Type		3			3			3			3	
Unit Extension		2.0			2.0			2.0			2.0	
Ped/Bike/RTOR Volume	0	0	1	0	0	1	0	0	2	0	0	6
Lane Width		12.0			12.0			12.0			12.0	
Parking/Grade/Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking/Hour												
Bus Stops/Hour		0			0			0			0	
Minimum Pedestrian Time												
Phasing	EW Perm	02	03	04	NS Perm	06	07	08				
Timing	G = 30.0	G =	G =	G =	G = 20.0	G =	G =	G =				
	Y = 7	Y =	Y =	Y =	Y = 7	Y =	Y =	Y =				
Duration of Analysis (hrs) = 0.25							Cycle Length C = 64.0					

Lane Group Capacity, Control Delay, and LOS Determination												
	EB			WB			NB			SB		
Adjusted Flow Rate		328			266			152			201	
Lane Group Capacity		852			919			618			565	
v/c Ratio		0.38			0.29			0.25			0.36	
Green Ratio		0.52			0.52			0.36			0.36	
Uniform Delay d <sub>1</sub>		9.4			8.8			14.4			15.1	
Delay Factor k		0.04			0.04			0.04			0.04	
Incremental Delay d <sub>2</sub>		0.1			0.1			0.1			0.1	
PF Factor		1.000			1.000			1.000			1.000	
Control Delay		9.5			8.9			14.5			15.2	
Lane Group LOS		A			A			B			B	
Approach Delay	9.5			8.9			14.5			15.2		
Approach LOS	A			A			B			B		
Intersection Delay	11.3			Intersection LOS						B		

# SHORT REPORT

General Information					Site Information				
Analyst	nbm np1				Intersection	Woodbine-Ocean/Corsons Tavern			
Agency or Co.	Shropshire Associates LLC				Area Type	All other areas			
Date Performed	4/13/2007				Jurisdiction	Cape May County			
Time Period	No-Build PM Peak Hour				Analysis Year	2009			

## Volume and Timing Input

		EB			WB			NB			SB				
		LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT		
Number of Lanes		0	1	0	0	1	0	0	1	0	0	1	0		
Lane Group			LTR			LTR			LTR			LTR			
Volume (vph)		97	241	15	19	324	52	11	101	23	43	79	80		
% Heavy Vehicles		2	2	2	2	2	2	2	2	2	2	2	2		
PHF		0.81	0.81	0.81	0.90	0.90	0.90	0.74	0.74	0.74	0.91	0.91	0.91		
Pretimed/Actuated (P/A)		A	A	A	A	A	A	A	A	A	A	A	A		
Startup Lost Time			2.0			2.0			2.0			2.0			
Extension of Effective Green			5.0			5.0			5.0			5.0			
Arrival Type			3			3			3			3			
Unit Extension			2.0			2.0			2.0			2.0			
Ped/Bike/RTOR Volume		0	0	2	0	0	5	0	0	2	0	0	8		
Lane Width			12.0			12.0			12.0			12.0			
Parking/Grade/Parking		N	0	N	N	0	N	N	0	N	N	0	N		
Parking/Hour															
Bus Stops/Hour			0			0			0			0			
Minimum Pedestrian Time															
Phasing	EW Perm	02		03		04		NS Perm		06		07		08	
Timing	G = 30.0	G =		G =		G =		G = 20.0		G =		G =		G =	
	Y = 7	Y =		Y =		Y =		Y = 7		Y =		Y =		Y =	
Duration of Analysis (hrs) = 0.25								Cycle Length C = 64.0							

## Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
Adjusted Flow Rate		434			433			179			213	
Lane Group Capacity		758			917			634			574	
v/c Ratio		0.57			0.47			0.28			0.37	
Green Ratio		0.52			0.52			0.36			0.36	
Uniform Delay d <sub>1</sub>		10.7			9.9			14.6			15.2	
Delay Factor k		0.11			0.04			0.04			0.04	
Incremental Delay d <sub>2</sub>		0.7			0.1			0.1			0.1	
PF Factor		1.000			1.000			1.000			1.000	
Control Delay		11.3			10.1			14.7			15.3	
Lane Group LOS		B			B			B			B	
Approach Delay		11.3			10.1			14.7			15.3	
Approach LOS		B			B			B			B	
Intersection Delay		12.0			Intersection LOS						B	

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information	
Analyst	<i>nbm na2</i>		Intersection	<i>Woodbine-Ocean View/East Acces</i>
Agency/Co.	<i>Shropshire Associates LLC</i>		Jurisdiction	<i>Cape May County</i>
Date Performed	<i>4/13/2007</i>		Analysis Year	<i>2009</i>
Analysis Time Period	<i>No-Build AM Peak Hour</i>			

Project Description *7047 - R.E. Pierson*

East/West Street: *Woodbine-Ocean View (CR 550)*

North/South Street: *East Site Access*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	0	268			237	8
Peak-Hour Factor, PHF	0.90	0.90	1.00	1.00	0.77	0.77
Hourly Flow Rate, HFR (veh/h)	0	297	0	0	307	10
Percent Heavy Vehicles	2	—	—	0	—	—
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	<i>LT</i>					<i>TR</i>
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				8		0
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.50	1.00	0.50
Hourly Flow Rate, HFR (veh/h)	0	0	0	16	0	0
Percent Heavy Vehicles	0	0	0	2	0	2
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					<i>LR</i>	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LR</i>	
v (veh/h)	0						16	
C (m) (veh/h)	1243						458	
v/c	0.00						0.03	
95% queue length	0.00						0.11	
Control Delay (s/veh)	7.9						13.1	
LOS	<i>A</i>						<i>B</i>	
Approach Delay (s/veh)	--	--				13.1		
Approach LOS	--	--				<i>B</i>		

## TWO-WAY STOP CONTROL SUMMARY

### General Information

Analyst *nbm np2*  
 Agency/Co. *Shropshire Associates LLC*  
 Date Performed *4/13/2007*  
 Analysis Time Period *No-Build PM Peak Hour*

### Site Information

Intersection *Woodbine-Ocean View/East Acces*  
 Jurisdiction *Cape May County*  
 Analysis Year *2009*

Project Description *7047 - R.E. Pierson*

East/West Street: *Woodbine-Ocean View (CR 550)*

North/South Street: *East Site Access*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	0	353			406	9
Peak-Hour Factor, PHF	0.78	0.78	1.00	1.00	0.92	0.92
Hourly Flow Rate, HFR (veh/h)	0	452	0	0	441	9
Percent Heavy Vehicles	2	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	LT					TR
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				0		2
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.50	1.00	0.50
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	4
Percent Heavy Vehicles	0	0	0	2	0	2
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					LR	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	0						4	
C (m) (veh/h)	1110						612	
v/c	0.00						0.01	
95% queue length	0.00						0.02	
Control Delay (s/veh)	8.2						10.9	
LOS	A						B	
Approach Delay (s/veh)	--	--					10.9	
Approach LOS	--	--					B	

## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information	
Analyst	<i>nbm na3</i>		Intersection	<i>Woodbine-Ocean View/West Acces</i>
Agency/Co.	<i>Shropshire Associates LLC</i>		Jurisdiction	<i>Cape May County</i>
Date Performed	<i>4/13/2007</i>		Analysis Year	<i>2009</i>
Analysis Time Period	<i>No-Build AM Peak Hour</i>			

Project Description *7047 - R.E. Pierson*

East/West Street: *Woodbine-Ocean View (CR 550)*

North/South Street: *West Site Access*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	6	268			237	0
Peak-Hour Factor, PHF	0.90	0.90	1.00	1.00	0.77	0.77
Hourly Flow Rate, HFR (veh/h)	6	297	0	0	307	0
Percent Heavy Vehicles	2	—	—	0	—	—
Median Type	<i>Undivided</i>					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	<i>LT</i>					<i>TR</i>
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				1		0
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.25	1.00	0.25
Hourly Flow Rate, HFR (veh/h)	0	0	0	4	0	0
Percent Heavy Vehicles	0	0	0	2	0	2
Percent Grade (%)	0			0		
Flared Approach		<i>N</i>			<i>N</i>	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					<i>LR</i>	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	<i>LT</i>						<i>LR</i>	
v (veh/h)	6						4	
C (m) (veh/h)	1254						452	
v/c	0.00						0.01	
95% queue length	0.01						0.03	
Control Delay (s/veh)	7.9						13.0	
LOS	<i>A</i>						<i>B</i>	
Approach Delay (s/veh)	—	—					13.0	
Approach LOS	—	—					<i>B</i>	



## TWO-WAY STOP CONTROL SUMMARY

General Information			Site Information	
Analyst	nbn np3		Intersection	Woodbine-Ocean View/West Acces
Agency/Co.	Shropshire Associates LLC		Jurisdiction	Cape May County
Date Performed	4/13/2007		Analysis Year	2009
Analysis Time Period	No-Build PM Peak Hour			

Project Description 7047 - R.E. Pierson

East/West Street: Woodbine-Ocean View (CR 550)

North/South Street: West Site Access

Intersection Orientation: East-West

Study Period (hrs): 0.25

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	0	353			408	0
Peak-Hour Factor, PHF	0.78	0.78	1.00	1.00	0.92	0.92
Hourly Flow Rate, HFR (veh/h)	0	452	0	0	443	0
Percent Heavy Vehicles	2	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	LT					TR
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				0		2
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.50	1.00	0.50
Hourly Flow Rate, HFR (veh/h)	0	0	0	0	0	4
Percent Heavy Vehicles	0	0	0	2	0	2
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					LR	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	0						4	
C (m) (veh/h)	1117						615	
v/c	0.00						0.01	
95% queue length	0.00						0.02	
Control Delay (s/veh)	8.2						10.9	
LOS	A						B	
Approach Delay (s/veh)	--	--					10.9	
Approach LOS	--	--					B	

# SHORT REPORT

## General Information

Analyst nbm ba1  
 Agency or Co. Shropshire Associates LLC  
 Date Performed 4/13/2007  
 Time Period Build AM Peak Hour

## Site Information

Intersection Woodbine-Ocean/Corsons  
 Tavern  
 Area Type All other areas  
 Jurisdiction Cape May County  
 Analysis Year 2009

## Volume and Timing Input

		EB			WB			NB			SB				
		LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT		
Number of Lanes		0	1	0	0	1	0	0	1	0	0	1	0		
Lane Group			LTR			LTR			LTR			LTR			
Volume (vph)		57	222	6	15	181	13	15	63	18	34	44	65		
% Heavy Vehicles		2	2	2	2	2	2	2	2	2	2	2	2		
PHF		0.84	0.84	0.84	0.74	0.74	0.74	0.62	0.62	0.62	0.67	0.67	0.67		
Pretimed/Actuated (P/A)		A	A	A	A	A	A	A	A	A	A	A	A		
Startup Lost Time			2.0			2.0			2.0			2.0			
Extension of Effective Green			5.0			5.0			5.0			5.0			
Arrival Type			3			3			3			3			
Unit Extension			2.0			2.0			2.0			2.0			
Ped/Bike/RTOR Volume		0	0	1	0	0	1	0	0	2	0	0	7		
Lane Width			12.0			12.0			12.0			12.0			
Parking/Grade/Parking		N	0	N	N	0	N	N	0	N	N	0	N		
Parking/Hour															
Bus Stops/Hour			0			0			0			0			
Minimum Pedestrian Time															
Phasing	EW Perm	02		03		04		NS Perm		06		07		08	
Timing	G = 30.0	G =		G =		G =		G = 20.0		G =		G =		G =	
	Y = 7	Y =		Y =		Y =		Y = 7		Y =		Y =		Y =	
Duration of Analysis (hrs) = 0.25								Cycle Length C = 64.0							

## Lane Group Capacity, Control Delay, and LOS Determination

	EB			WB			NB			SB		
Adjusted Flow Rate	338			281			152			204		
Lane Group Capacity	850			920			612			565		
v/c Ratio	0.40			0.31			0.25			0.36		
Green Ratio	0.52			0.52			0.36			0.36		
Uniform Delay $d_1$	9.4			8.9			14.4			15.1		
Delay Factor k	0.04			0.04			0.04			0.04		
Incremental Delay $d_2$	0.1			0.1			0.1			0.1		
PF Factor	1.000			1.000			1.000			1.000		
Control Delay	9.6			9.0			14.5			15.2		
Lane Group LOS	A			A			B			B		
Approach Delay	9.6			9.0			14.5			15.2		
Approach LOS	A			A			B			B		
Intersection Delay	11.3			Intersection LOS						B		

# SHORT REPORT

## General Information

Analyst *nbm bp1*  
 Agency or Co. *Shropshire Associates LLC*  
 Date Performed *4/13/2007*  
 Time Period *Build PM Peak Hour*

## Site Information

Intersection *Woodbine-Ocean/Corsons Tavern*  
 Area Type *All other areas*  
 Jurisdiction *Cape May County*  
 Analysis Year *2009*

## Volume and Timing Input

	EB			WB			NB			SB		
	LT	TH	RT	LT	TH	RT	LT	TH	RT	LT	TH	RT
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Lane Group		LTR			LTR			LTR			LTR	
Volume (vph)	98	252	15	19	335	52	11	101	23	43	79	81
% Heavy Vehicles	2	2	2	2	2	2	2	2	2	2	2	2
PHF	0.81	0.81	0.81	0.90	0.90	0.90	0.74	0.74	0.74	0.91	0.91	0.91
Pretimed/Actuated (P/A)	A	A	A	A	A	A	A	A	A	A	A	A
Startup Lost Time		2.0			2.0			2.0			2.0	
Extension of Effective Green		5.0			5.0			5.0			5.0	
Arrival Type		3			3			3			3	
Unit Extension		2.0			2.0			2.0			2.0	
Ped/Bike/RTOR Volume	0	0	2	0	0	5	0	0	2	0	0	9
Lane Width		12.0			12.0			12.0			12.0	
Parking/Grade/Parking	N	0	N	N	0	N	N	0	N	N	0	N
Parking/Hour												
Bus Stops/Hour		0			0			0			0	
Minimum Pedestrian Time												

Phasing	EW Perm	02	03	04	NS Perm	06	07	08
Timing	G = 30.0	G =	G =	G =	G = 20.0	G =	G =	G =
	Y = 7	Y =	Y =	Y =	Y = 7	Y =	Y =	Y =
Duration of Analysis (hrs) = 0.25						Cycle Length C = 64.0		

## Lane Group Capacity, Control Delay, and LOS Determination

	EB		WB		NB		SB	
Adjusted Flow Rate	448		445		179		213	
Lane Group Capacity	756		918		634		574	
v/c Ratio	0.59		0.48		0.28		0.37	
Green Ratio	0.52		0.52		0.36		0.36	
Uniform Delay $d_1$	10.8		10.0		14.6		15.2	
Delay Factor k	0.13		0.04		0.04		0.04	
Incremental Delay $d_2$	0.9		0.1		0.1		0.1	
PF Factor	1.000		1.000		1.000		1.000	
Control Delay	11.7		10.2		14.7		15.3	
Lane Group LOS	B		B		B		B	
Approach Delay	11.7		10.2		14.7		15.3	
Approach LOS	B		B		B		B	
Intersection Delay	12.2		Intersection LOS				B	

## TWO-WAY STOP CONTROL SUMMARY

### General Information

Analyst *nbm ba2*  
 Agency/Co. *Shropshire Associates LLC*  
 Date Performed *4/13/2007*  
 Analysis Time Period *Build AM Peak Hour*

### Site Information

Intersection *Woodbine-Ocean View/East Acces*  
 Jurisdiction *Cape May County*  
 Analysis Year *2009*

Project Description *7047 - R.E. Pierson*

East/West Street: *Woodbine-Ocean View (CR 550)*

North/South Street: *East Site Access*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	8	272			244	17
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.74	0.74
Hourly Flow Rate, HFR (veh/h)	8	302	0	0	329	22
Percent Heavy Vehicles	75	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	LT					TR
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				13		5
Peak-Hour Factor, PHF	0.90	0.90	0.90	0.90	0.90	0.90
Hourly Flow Rate, HFR (veh/h)	0	0	0	14	0	5
Percent Heavy Vehicles	0	0	0	75	0	75
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					LR	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	8						19	
C (m) (veh/h)	895						371	
v/c	0.01						0.05	
95% queue length	0.03						0.16	
Control Delay (s/veh)	9.1						15.2	
LOS	A						C	
Approach Delay (s/veh)	--	--				15.2		
Approach LOS	--	--				C		

## TWO-WAY STOP CONTROL SUMMARY

### General Information

Analyst *nbm bp2*  
 Agency/Co. *Shropshire Associates LLC*  
 Date Performed *4/13/2007*  
 Analysis Time Period *Build PM Peak Hour*

### Site Information

Intersection *Woodbine-Ocean View/East Acces*  
 Jurisdiction *Cape May County*  
 Analysis Year *2009*

Project Description *7047 - R.E. Pierson*

East/West Street: *Woodbine-Ocean View (CR 550)*

North/South Street: *East Site Access*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	7	359			412	15
Peak-Hour Factor, PHF	0.78	0.78	1.00	1.00	0.92	0.92
Hourly Flow Rate, HFR (veh/h)	8	460	0	0	447	16
Percent Heavy Vehicles	75	—	—	0	—	—
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	LT					TR
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				6		9
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.90	1.00	0.90
Hourly Flow Rate, HFR (veh/h)	0	0	0	6	0	10
Percent Heavy Vehicles	0	0	0	75	0	75
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					LR	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	8						16	
C (m) (veh/h)	803						332	
v/c	0.01						0.05	
95% queue length	0.03						0.15	
Control Delay (s/veh)	9.5						16.4	
LOS	A						C	
Approach Delay (s/veh)	—	—					16.4	
Approach LOS	—	—					C	

## TWO-WAY STOP CONTROL SUMMARY

### General Information

Analyst *nbm ba3*  
 Agency/Co. *Shropshire Associates LLC*  
 Date Performed *4/13/2007*  
 Analysis Time Period *Build AM Peak Hour*

### Site Information

Intersection *Woodbine-Ocean View/West Acces*  
 Jurisdiction *Cape May County*  
 Analysis Year *2009*

Project Description *7047 - R.E. Pierson*

East/West Street: *Woodbine-Ocean View (CR 550)*

North/South Street: *West Site Access*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	14	276			242	7
Peak-Hour Factor, PHF	0.90	0.90	1.00	1.00	0.77	0.77
Hourly Flow Rate, HFR (veh/h)	15	306	0	0	314	9
Percent Heavy Vehicles	75	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	LT					TR
Upstream Signal		0			0	

Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				4		6
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.90	1.00	0.90
Hourly Flow Rate, HFR (veh/h)	0	0	0	4	0	6
Percent Heavy Vehicles	0	0	0	75	0	75
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					LR	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	15						10	
C (m) (veh/h)	919						446	
v/c	0.02						0.02	
95% queue length	0.05						0.07	
Control Delay (s/veh)	9.0						13.3	
LOS	A						B	
Approach Delay (s/veh)	--	--					13.3	
Approach LOS	--	--					B	

## TWO-WAY STOP CONTROL SUMMARY

### General Information

Analyst *nbm bp3*  
 Agency/Co. *Shropshire Associates LLC*  
 Date Performed *4/13/2007*  
 Analysis Time Period *Build PM Peak Hour*

### Site Information

Intersection *Woodbine-Ocean View/West Access*  
 Jurisdiction *Cape May County*  
 Analysis Year *2009*

Project Description *7047 - R.E. Pierson*

East/West Street: *Woodbine-Ocean View (CR 550)*

North/South Street: *West Site Access*

Intersection Orientation: *East-West*

Study Period (hrs): *0.25*

### Vehicle Volumes and Adjustments

Major Street	Eastbound			Westbound		
Movement	1	2	3	4	5	6
	L	T	R	L	T	R
Volume (veh/h)	9	360			415	6
Peak-Hour Factor, PHF	0.78	0.78	1.00	1.00	0.92	0.92
Hourly Flow Rate, HFR (veh/h)	11	461	0	0	451	6
Percent Heavy Vehicles	75	--	--	0	--	--
Median Type	Undivided					
RT Channelized			0			0
Lanes	0	1	0	0	1	0
Configuration	LT					TR
Upstream Signal		0			0	
Minor Street	Northbound			Southbound		
Movement	7	8	9	10	11	12
	L	T	R	L	T	R
Volume (veh/h)				6		11
Peak-Hour Factor, PHF	1.00	1.00	1.00	0.90	1.00	0.90
Hourly Flow Rate, HFR (veh/h)	0	0	0	6	0	12
Percent Heavy Vehicles	0	0	0	75	0	75
Percent Grade (%)	0			0		
Flared Approach		N			N	
Storage		0			0	
RT Channelized			0			0
Lanes	0	0	0	0	0	0
Configuration					LR	

### Delay, Queue Length, and Level of Service

Approach	Eastbound	Westbound	Northbound			Southbound		
Movement	1	4	7	8	9	10	11	12
Lane Configuration	LT						LR	
v (veh/h)	11						18	
C (m) (veh/h)	807						342	
v/c	0.01						0.05	
95% queue length	0.04						0.17	
Control Delay (s/veh)	9.5						16.1	
LOS	A						C	
Approach Delay (s/veh)	--	--				16.1		
Approach LOS	--	--				C		